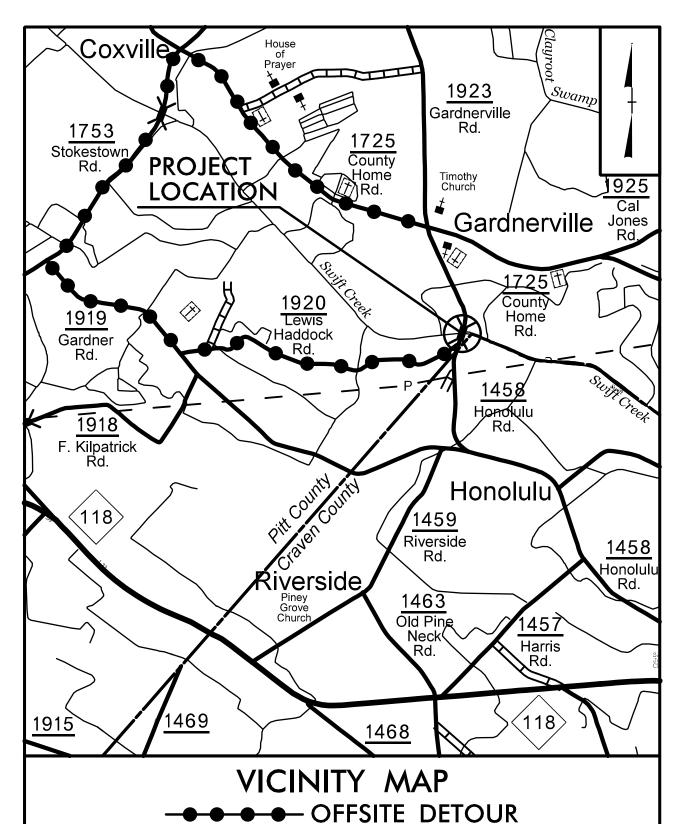
See Sheet 1A For Index of Sheets See Sheet 1B for Conventional Symbols

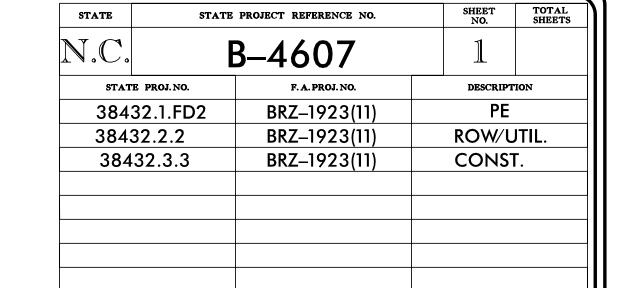


STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

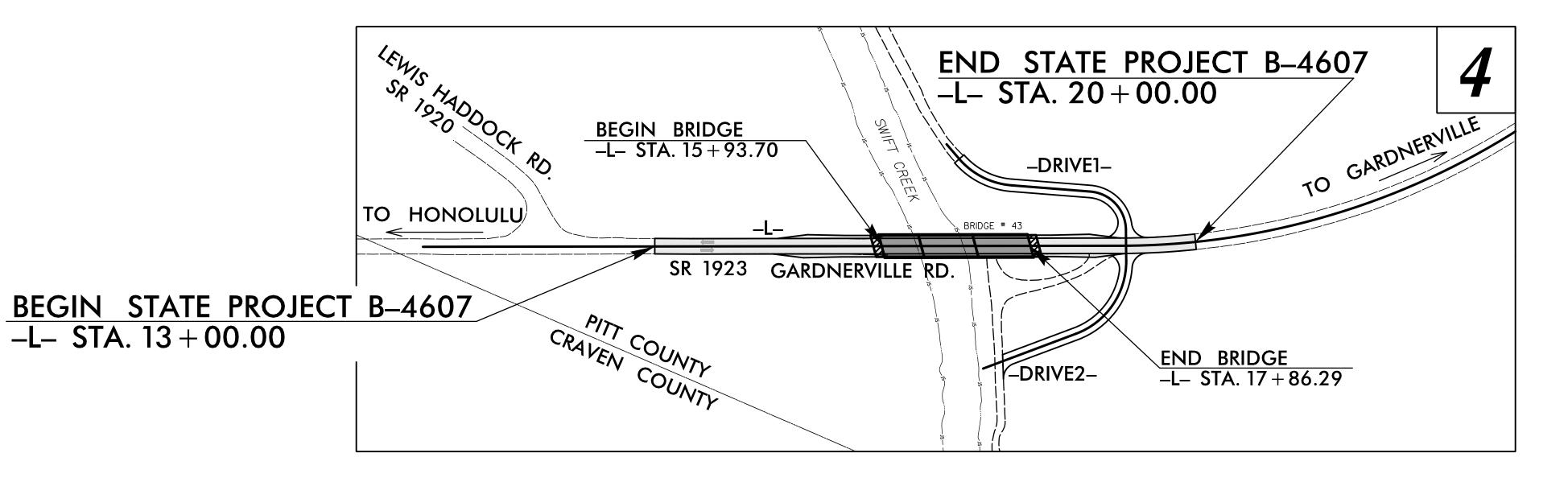
PITT COUNTY

LOCATION: REPLACE BRIDGE NO. 43 ON SR 1923 OVER SWIFT CREEK

TYPE OF WORK: GRADING, DRAINAGE, STRUCTURE AND PAVING







CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II. THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES PLANS PROFILE (HORIZONTAL) SUB-REGIONAL TIER DESIGN STANDARDS PROFILE (VERTICAL)

DESIGN DATA

ADT 2019 = 650ADT 2040 = 1060T = 6%

V = 55 MPH

FUNCT CLASS = RURAL LOCAL

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4607 0.096 mile LENGTH STRUCTURES TIP PROJECT B-4607 0.037 mile TOTAL LENGTH TIP PROJECT B-4607 = 0.133 mile

Prepared For: **DIVISION OF HIGHWAYS** 1037 W. H. Smith Blvd, Greenville NC, 27834 TGS ENGINEERS **PH** (919) 733–8887 706 HILLSBOROUGH ST. CORP. LICENSE NO.: SUITE 200

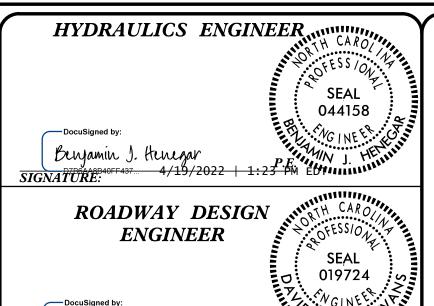
RALEIGH, NC 27603

2018 STANDARD SPECIFICATIONS RIGHT OF WAY DATE: OCTOBER 29, 2021

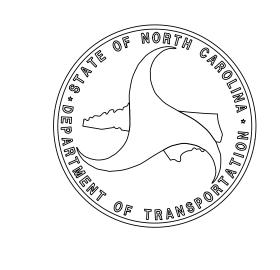
> LETTING DATE: MAY 25, 2022

BURKE EVANS, P.E. PROJECT ENGINEER

MICHAEL AMAN, PE PROJECT ENGINEER NCDOT DIVISION 2



David Burke Evans



PROJECT REFERENCE NO. SHEET NO. IA

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ROADWAY DESIGN
ENGINEER

SEAL
019724

Double of ESSION

Double of ESSION

SEAL
019724

Double of ESSION

Double of ESSIO

INDEX OF SHEETS

| SHEET NUMBER | SHEET |
|----------------------|---|
| 1 | TITLE SHEET |
| 1A | INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS |
| 1B | CONVENTIONAL SYMBOLS |
| 2A-1 | PAVEMENT SCHEDULE, TYPICAL SECTIONS, DETAIL FOR SHOULDER BERM GUTTER, DETAIL SHOWING METHOD OF WEDGING, AND INCIDENTAL MILLING DETAIL |
| 2C-1 THRU 2C-3 | DETAIL SHEETS FOR MODIFIED RAIL SECTIONS, TYPE III UNIT, AND AT-1 UNIT |
| 2D-1 | PREFORMED SCOUR HOLE |
| 3B–1 | SUMMARY OF EARTHWORK, PAVEMENT REMOVAL SUMMARY, SUMMARY OF SHOULDER BERM GUTTER, & GUARDRAIL SUMMARY |
| 3D-1 | DRAINAGE SUMMARY |
| 3G–1 | GEOTECHNICAL SUMMARY |
| 4 | PLAN SHEET |
| 5 | PROFILE SHEET |
| RW02C-1 THRU RW02C-2 | RIGHT OF WAY SHEETS |
| TMP-1 THRU TMP-2 | TRANSPORTATION MANAGEMENT PLANS |
| EC-1 THRU EC-5 | EROSION CONTROL PLANS |
| RF–1 | REFORESTATION DETAIL SHEET |
| X–1A | CROSS SECTION SUMMARY SHEET |
| X-1 THRU X-8 | CROSS SECTIONS |

STRUCTURE PLANS

GENERAL NOTES

GENERAL NOTES: 2018 SPECIFICATIONS EFFECTIVE: 01–16–2018 REVISED:

DE LINE:

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY

METHOD II.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF

SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

SIDE ROADS:

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

SUBSURFACE DRAINS:

SUBSURFACE DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 815.02 AT

LOCATIONS DIRECTED BY THE ENGINEER.

DRIVEWAYS:

DRIVEWAYS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. 848.03
AT LOCATIONS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

STRUCTURE SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS—SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

STANDARD DRAWINGS

2018 ROADWAY ENGLISH STANDARD DRAWINGS

EFF. 01–16–2018

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch – N. C. Department of Transportation – Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

DIVISION 2 – EARTHWORK

200.02 Method of Clearing – Method II

25.02 Guide for Grading Subgrade – Secondary and Local
25.04 Method of Obtaining Superelevation – Two Lane Pavement

DIVISION 3 – PIPE CULVERTS
300.01 Method of Pipe Installation
310.10 Driveway Pipe Construction

DIVISION 4 - MAJOR STRUCTURES

422.02 Bridge Approach Fills – Type II Modified Approach Fill

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

560.01 Method of Shoulder Construction – High Side of Superelevated Curve – Method I

DIVISION 8 – INCIDENTALS

5.02 Subsurface Drain

840.00 Concrete Base Pad for Drainage Structures

840.25 Anchorage for Frames – Brick or Concrete or Precast

40.29 Frames and Narrow Slot Flat Grates

840.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Grates

840.46 Traffic Bearing Precast Drainage Structures

840.66 Drainage Structure Steps

846.01 Concrete Curb, Gutter and Curb & Gutter 846.04 Drop Inlet Installation in Shoulder Berm Gutter

862.01 Guardrail Placement

862.01 Guardrail Placeme

862.02 Guardrail Installation

862.03 Structure Anchor Units 876.02 Guide for Rip Rap at Pipe Outlets

S-1 THRU S-24

| CONV | | , | | SYMBOLS | • |
|------|--|------------|--|----------|---|
| | | (OLINA, DI | | HIGHWAYS | |

| BOUNDARIES AND PROPERTY | <i>7.</i> | RAILROADS: Note: Not to | Scale *S.U.E. |
|--|--|---|---|
| State Line — | | | |
| County Line | | Standard Gauge | CSX TRANSPORTATION • • • • • • • • • • • • • • • • • • • |
| Township Line | | RR Signal Milepost | MILEPOST 35 |
| City Line | | Switch | SWITCH |
| Reservation Line | | RR Abandoned | |
| Property Line | | RR Dismantled | <i>EX</i> |
| Existing Iron Pin | <u></u> | | MAJO |
| Computed Property Corner | × | RIGHT OF WAY & PROJECT C | ONTROL: Brid |
| Property Monument | | Secondary Horiz and Vert Control Point —— | Brid |
| Parcel/Sequence Number ———— | | Primary Horiz Control Point | MINO |
| Existing Fence Line | • | Primary Horiz and Vert Control Point | . Hed |
| Proposed Woven Wire Fence | | Exist Permanent Easment Pin and Cap | Pipe |
| Proposed Chain Link Fence | | New Permanent Easement Pin and Cap — | Foo |
| Proposed Barbed Wire Fence | | Vertical Benchmark | Dra |
| Existing Wetland Boundary | | Existing Right of Way Marker ———— | Pav |
| Proposed Wetland Boundary | | Existing Right of Way Line | Stoi |
| Existing Endangered Animal Boundary —— | | New Right of Way Line | Stor |
| Existing Endangered Plant Boundary | | New Right of Way Line with Pin and Cap— | |
| Existing Litatingered Flam Boundary Existing Historic Property Boundary | —————————————————————————————————————— | | 9 |
| . , , , | | New Right of Way Line with Concrete or Granite R/W Marker | POW |
| Known Contamination Area: Soil | | New Control of Access Line with | Exis |
| Potential Contamination Area: Soil | | Concrete C/A Marker | Pro |
| Known Contamination Area: Water | | Existing Control of Access | |
| Potential Contamination Area: Water | | New Control of Access | Pro |
| Contaminated Site: Known or Potential — | | Existing Easement Line ———————————————————————————————————— | EPow |
| BUILDINGS AND OTHER CUL | TURE: | New Temporary Construction Easement – | Pow |
| Gas Pump Vent or U/G Tank Cap | <u> </u> | New Temporary Drainage Easement —— | TDE Pow |
| Sign — | <u> </u> | New Permanent Drainage Easement —— | PDE U/G |
| Well — | W | New Permanent Drainage / Utility Easement | ——— DUE———— H—F |
| Small Mine | <u></u> | New Permanent Utility Easement ——— | PUE U/G |
| Foundation ———————————————————————————————————— | | New Temporary Utility Easement | TUE U/G |
| Area Outline | | New Aerial Utility Easement | U/G |
| Cemetery | | | TELE |
| Building — | | ROADS AND RELATED FEATUR | RES: |
| School — | | Existing Edge of Pavement | Exis |
| Church — | — <u>_</u> | Existing Curb ———— | Pro |
| Dam — | | Proposed Slope Stakes Cut ———— | <u>C</u> |
| HYDROLOGY: | | Proposed Slope Stakes Fill ———— | <u>F</u> |
| Stream or Body of Water ———————————————————————————————————— | | Proposed Curb Ramp | CR |
| Hydro, Pool or Reservoir — | | Existing Metal Guardrail | U/(- |
| Jurisdictional Stream | Js | Proposed Guardrail ———————————————————————————————————— | 1 1/1 - |
| Buffer Zone 1 | BZ 1 | Existing Cable Guiderail | U/C ₂ |
| Buffer Zone 2 | BZ 2 | Proposed Cable Guiderail | 1 1/6 - |
| Flow Arrow | | Equality Symbol | U/G |
| Disappearing Stream ———————————————————————————————————— | > | Pavement Removal | I]/(- |
| Spring — | -0 | | U/G |
| Wetland | <u> </u> | VEGETATION: | U/G |
| Proposed Lateral, Tail, Head Ditch ———— | FLOW | Single Tree | − ຜ: U/G |
| False Sump — | _ | Single Shrub | – |

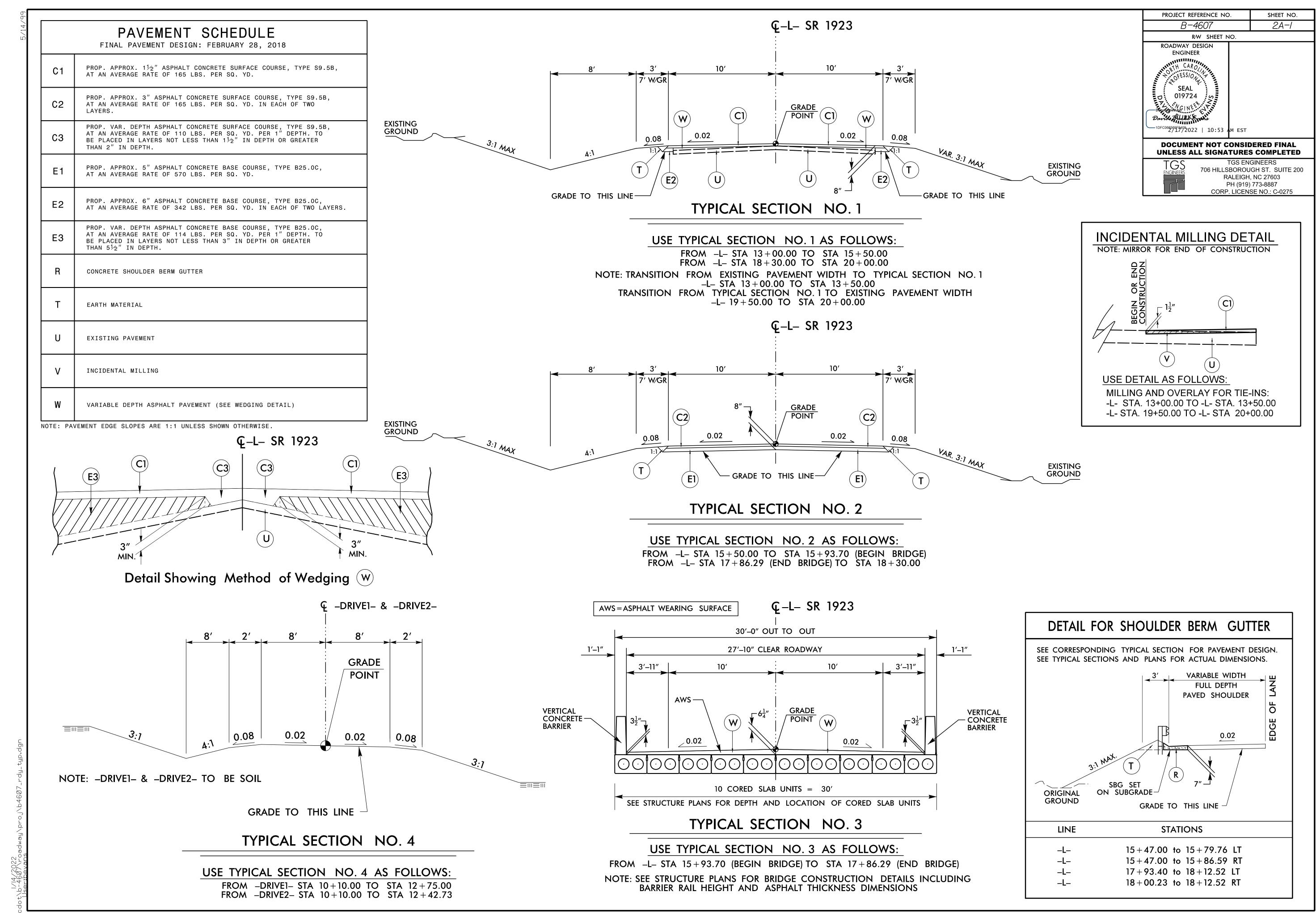
| Hedge — | | Water Manhole |
|--|----------------|--|
| Woods Line ———————————————————————————————————— | | Water Meter |
| Orchard — | | Water Valve |
| | | Water Hydrant |
| /ineyard | — Vineyard | U/G Water Line LOS B (S.U.E*) |
| EXISTING STRUCTURES: | | U/G Water Line LOS C (S.U.E*) |
| AAJOR: | | U/G Water Line LOS D (S.U.E*) |
| Bridge, Tunnel or Box Culvert | | Above Ground Water Line |
| Bridge Wing Wall, Head Wall and End Wall | -) CONC WW [| T) / |
| MINOR: Head and End Wall —————————————————————————————————— | | TV: TV Pedestal |
| | | TV Tower |
| Pipe Culvert — Footbridge — Foo | | U/G TV Cable Hand Hole |
| rootbridge ———————————————————————————————————— | / | U/G TV Cable LOS B (S.U.E.*) |
| Drainage Box: Catch Basin, DI or JB | | U/G TV Cable LOS C (S.U.E.*) |
| Paved Ditch Gutter | | U/G TV Cable LOS D (S.U.E.*) |
| Storm Sewer Manhole ———————————————————————————————————— | | U/G Fiber Optic Cable LOS B (S.U.E.*) |
| Storm Sewer — | s | U/G Fiber Optic Cable LOS C (S.U.E.*) |
| UTILITIES: | | U/G Fiber Optic Cable LOS D (S.U.E.*) |
| OWER: | | |
| Existing Power Pole ———————————————————————————————————— | _ • | GAS: |
| Proposed Power Pole | | Gas Valve |
| Existing Joint Use Pole | 1 | Gas Meter |
| Proposed Joint Use Pole | ı | U/G Gas Line LOS B (S.U.E.*) |
| Power Manhole | | U/G Gas Line LOS C (S.U.E.*) |
| Power Line Tower | | U/G Gas Line LOS D (S.U.E.*) |
| Power Transformer | | Above Ground Gas Line |
| U/G Power Cable Hand Hole | | SANITARY SEWER: |
| H-Frame Pole | | Sanitary Sewer Manhole |
| U/G Power Line LOS B (S.U.E.*) | | Sanitary Sewer Cleanout |
| U/G Power Line LOS C (S.U.E.*) | | U/G Sanitary Sewer Line —————— |
| U/G Power Line LOS D (S.U.E.*) | | Above Ground Sanitary Sewer — |
| | | SS Forced Main Line LOS B (S.U.E.*) — |
| ELEPHONE: | | SS Forced Main Line LOS C (S.U.E.*) — |
| Existing Telephone Pole ———————————————————————————————————— | | SS Forced Main Line LOS D (S.U.E.*)—— |
| Proposed Telephone Pole ———————————————————————————————————— | -0- | |
| Telephone Manhole | - ① | MISCELLANEOUS: |
| Telephone Pedestal ———————————————————————————————————— | - I | Utility Pole |
| Telephone Cell Tower ———————————————————————————————————— | - \ | Utility Pole with Base ———————————————————————————————————— |
| U/G Telephone Cable Hand Hole ———— | — НН | Utility Located Object ———————————————————————————————————— |
| U/G Telephone Cable LOS B (S.U.E.*) | | Utility Traffic Signal Box ——————————————————————————————————— |
| U/G Telephone Cable LOS C (S.U.E.*) | | Utility Unknown U/G Line LOS B (S.U.E.*) |
| U/G Telephone Cable LOS D (S.U.E.*) | | U/G Tank; Water, Gas, Oil ——————————————————————————————————— |
| U/G Telephone Conduit LOS B (S.U.E.*) —— | | Underground Storage Tank, Approx. Loc. — |
| U/G Telephone Conduit LOS C (S.U.E.*) | | A/G Tank; Water, Gas, Oil ————— |
| U/G Telephone Conduit LOS D (S.U.E.*)—— | | Geoenvironmental Boring ———————————————————————————————————— |
| U/G Fiber Optics Cable LOS B (S.U.E.*) —— | | U/G Test Hole LOS A (S.U.E.*) |
| U/G Fiber Optics Cable LOS C (S.U.E.*)—— | | Abandoned According to Utility Records — |
| U/G Fiber Optics Cable LOS D (S.U.E.*) | | End of Information ———————————————————————————————————— |

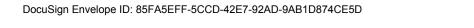
| WATER: | |
|---|--------------------|
| Water Manhole | - W |
| Water Meter | - 0 |
| Water Valve | - ⊗ |
| Water Hydrant | - ❖ |
| U/G Water Line LOS B (S.U.E*) | |
| U/G Water Line LOS C (S.U.E*) | |
| U/G Water Line LOS D (S.U.E*) | w |
| Above Ground Water Line | |
| ΓV: | |
| TV Pedestal ———————————————————————————————————— | - [] |
| TV Tower | - |
| U/G TV Cable Hand Hole | • |
| U/G TV Cable LOS B (S.U.E.*) | |
| U/G TV Cable LOS C (S.U.E.*) | |
| U/G TV Cable LOS D (S.U.E.*) | |
| U/G Fiber Optic Cable LOS B (S.U.E.*) | |
| | |
| U/G Fiber Optic Cable LOS C (S.U.E.*) | |
| U/G Fiber Optic Cable LOS D (S.U.E.*) | |
| GAS: | • |
| Gas Valve | · |
| Gas Meter — | · |
| U/G Gas Line LOS B (S.U.E.*) | |
| U/G Gas Line LOS C (S.U.E.*) | |
| U/G Gas Line LOS D (S.U.E.*) | |
| Above Ground Gas Line | A/G Gas |
| SANITARY SEWER: | |
| Sanitary Sewer Manhole | - |
| Sanitary Sewer Cleanout ———————————————————————————————————— | - |
| U/G Sanitary Sewer Line ————— | ss |
| Above Ground Sanitary Sewer — | A/G Sanitary Sewer |
| SS Forced Main Line LOS B (S.U.E.*) — | _ — — — FSS— — — |
| SS Forced Main Line LOS C (S.U.E.*) | FSS |
| SS Forced Main Line LOS D (S.U.E.*) | FSS |
| AICCELLANIECLIC. | |
| MISCELLANEOUS: Utility Pole ———————————————————————————————————— | _ |
| | _ |
| Utility Located Object | |
| Utility Located Object ———————————————————————————————————— | |
| Utility Traffic Signal Box ——————————————————————————————————— | |
| Utility Unknown U/G Line LOS B (S.U.E.*) | |
| U/G Tank; Water, Gas, Oil | |
| Underground Storage Tank, Approx. Loc. — | \ |
| A/G Tank; Water, Gas, Oil ——————————————————————————————————— | |
| Geoenvironmental Boring ————— | - |
| U/G Test Hole LOS A (S.U.E.*) | _ |

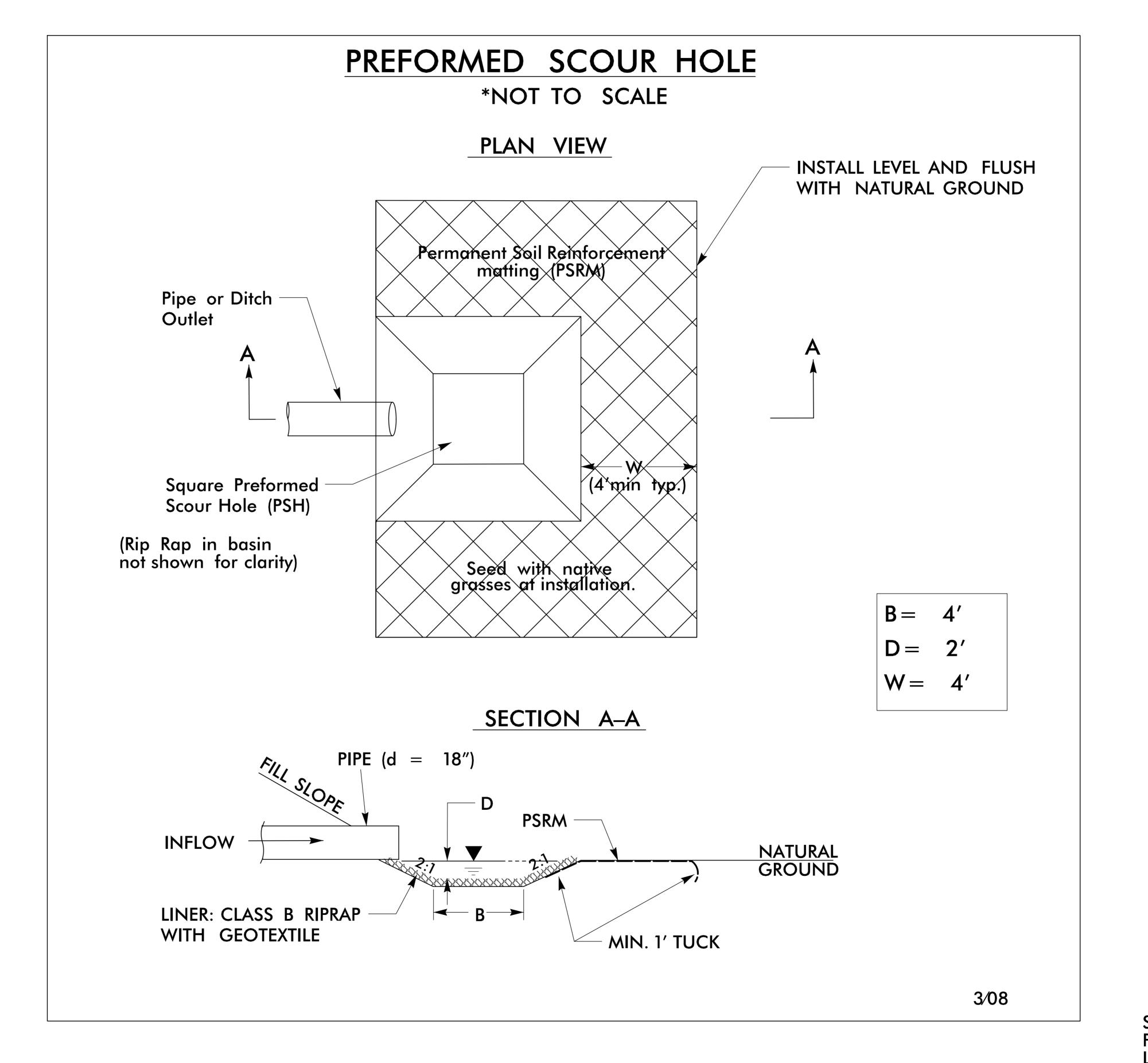
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E.O.I.

PROJECT REFERENCE NO. B-4607







PROJECT REFERENCE NO.

B-4607

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TGS FINE PROPERTY TO A POPULATION OF THE POPULAT

TGS ENGINEERS 706 HILLSBOROUGH ST. STE. 200 RALEIGH, NC 27603 PH (919) 773–8887 CORP. LICENSE NO.: C–0275

SEE PLAN VIEW FOR PREFORMED SCOUR HOLE LOCATIONS

 COMPUTED BY:
 DBE
 DATE:
 12 / 31 / 21

 CHECKED BY:
 DATE:

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

| PROJECT REFERENCE NO. | SHEET NO. |
|-----------------------|-----------|
| B-4607 | 3B-/ |

SUMMARY OF EARTHWORK

IN CUBIC YARDS

| LOCATION | UNCLASSIFIED EXCAVATION | UNDERCUT | EMBT+% | BORROW | WASTE |
|---|-------------------------|----------|--------|--------|-------|
| | | | | | |
| -L- 13+00.00 TO 15+93.70 (BEGIN BRIDGE) | 18 | | 224 | 206 | |
| | | | | | |
| -L- 17+86.29 (END BRIDGE) TO 20+00.00 | 519 | | 253 | | 266 |
| -DRIVE1- 10+10.00 TO 12+75.00 | 371 | | 251 | | 120 |
| | 517 | | | | |
| -DRIVE2- 10+10.00 TO 12+42.73 | 78 | | 255 | 177 | |
| | | | | | |
| SUBTOTAL | 986 | | 983 | 383 | 386 |
| | | | | | |
| WASTE TO REPLACE BORROW | | | | -383 | -383 |
| | | | | | |
| PROJECT TOTAL | 986 | | 983 | 0 | 3 |
| | | | | | |
| GRAND TOTAL | 986 | | | | |
| SAY | 1090 | | | | |

PER GEOTECHNICAL RECOMMENDATIONS:

ESTIMATED UNDERCUT = 750 CY (CONTINGENCY, AS DIRECTED BY THE ENGINEER)

ESTIMATED DRAINAGE DITCH EXCAVATION (DDE) = 50 CY

SELECT GRANULAR MATERIAL, CLASS II AND/OR CLASS III = 750 CY (CONTINGENCY, TO BE USED AS BACKFILL FOR UNDERCUT)

GEOTEXTILE FOR SOIL STABILIZATION = 800 SY (CONTINGENCY, AS DIRECTED BY THE ENGINEER)

APPROXIMATE QUANTITIES ONLY. CLEARING AND GRUBBING, UNCLASSIFIED EXCAVATION, FINE GRADING, AND REMOVAL OF EXISTING ASPHALT PAVEMENT WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR "GRADING".

NOTE: Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

PAVEMENT REMOVAL SUMMARY

IN SQUARE YARDS

| SURVEY LINE | STATION | STATION | LOCATION LT/RT/CL | SY |
|----------------|-------------------------|-------------------------|----------------------|-----|
| | | | | |
| -L- | 15 + 50 | 16 + 01.38 (EX. BRIDGE) | CL | 112 |
| | | | | |
| -L- | 17 + 72.39 (EX. BRIDGE) | 18 + 30 | CL | 124 |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | | |
| | | | TOTAL: | 236 |
| | | | | |
| | | | SAY: | 240 |

SUMMARY OF SHOULDER BERM GUTTER

IN LINEAR FEET

| STATION TO STATION | LOCATION | LF |
|-----------------------------------|----------|--------|
| _L_ STA. 15 + 47.00 to 15 + 79.76 | LT | 32.76 |
| -L- STA. 15 + 47.00 to 15 + 86.59 | RT | 39.59 |
| -L- STA. 17 + 93.40 to 18 + 12.52 | LT | 19.12 |
| -L- STA. 18+00.23 to 18+12.52 | RT | 12.29 |
| | | |
| | | |
| | TOTAL | 103.76 |
| | SAY | 105 |

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.
TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.
FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

GUARDRAIL SUMMARY

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.
G = GATING IMPACT ATTENUATOR TYPE 350
NG = NON-GATING IMPACT ATTENUATOR TYPE 350

| TEXIL ELITOTI | 1 Bioth a received | THE EAST SECTION S | T PARALLEL GUARDRA | 110 1110 01 | OUARDIVAIL. | | | | _ | _ | | | | | | | | | | - 11011 | | CI ATTENDATOR TITE 330 |
|---------------|--------------------|--------------------|--------------------|-------------|----------------|-----------------|-----------------|-----------------|----------------|-----------------|-----------------|-----------------|-----------------|-----------------|------|-------|-----------------------|----------------------------------|---|---------|--------------------|------------------------|
| SURVEY | DEC. CTA | 5\15\cdot 6\tau | LOCATION | | LENGTH | | WARRA | NT POINT | "N" DIST. | TOTAL | FLARE | LENGTH | w | | | ANCHO | RS | IMPACT ATTENUATOR TYPE 350 | | | REMOVE EXISTING | |
| LINE | BEG. STA. | END STA. | LOCATION | STRAIGHT | SHOP CURVED | DOUBLE FACED | APPROACH END | TRAILING END | FROM E.O.L. | SHOUL. WIDTH | APPROACH END | TRAILING END | APPROACH END | TRAILING END | TYF | PE I | GRAU 350 (TL–3) | TYPE 350 EA G NG | _ | | GUARDRAIL (LF) | REMARKS |
| -L- | 14 + 96.22 | 15 + 89.97 | LT | 93.75′ | | | | 15 + 89.97 | VAR. | VAR. | | 50′ | | 1′ | 1 | 1 | 1 | | | | | |
| -L- | 14 + 91.18 | 15 + 97.43 | RT | 106.25′ | | | 14 + 91.18 | | VAR. | VAR. | 50′ | | 1′ | | 1 | | 1 | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | |
| -L- | 17 + 82.56 | 18 + 63.81 | LT | 81.25′ | | | 17 + 82.56 | | VAR. | VAR. | 50′ | | 1′ | | 1 | | 1 | | | | | |
| -L- | 17 + 90.02 | 18 + 71.27 | RT | 81.25′ | | | | 17 + 90.02 | VAR. | VAR. | | 50′ | | 1′ | 1 | | 1 | | | | | |
| | | | SUBTOTAL (LF) | 362.50′ | | | | | | | | | TOTAL ANCH | HORS (EA) | 4 | 1 | 4 | | | | | |
| | | | LESS ANCHORS (LF) | 275′ | | | | | | | | | ANCHOR UNIT I | LENGTH (LF) | 18.7 | 75' | 50′ | | | | | |
| | | | TOTAL (LF) | 87.50′ | | | | ADDITIONAL C | | CTC. CAV 5 EA | | | DEDUCTION PE | R TYPE (LF) | 75 | 5′ | 200′ | | | | | |
| | _ | _ | SAY (LF) | 87.50′ | | | | ADDITIONAL G | JUARDRAIL PUS | DID: DAT D EA | ı | | TOTAL DEDUC | CTION (LF) | | 275′ | | | | | | |

| COMPUTED BY: | ВЈН | DATE: | 8/18/2021 |
|--------------|-----|-------|-----------|
| CHECKED BY: | ВЈН | DATE: | 8/18/2021 |

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

 Pitt County
 PROJECT NO.
 SHEET NO.

 Bridge #730043
 B-4607
 3D-1

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout.

| Column C | | | | | | | | | | · | | | | | | | | | | | | | | DWA | LLS | S, E ' | TC. | (FC | OR . | PIP | PES | 48 | INCH | ES o | & U | NDE | ER) | | | | | | | | | | | |
|--|----------------|------------------|------------|---------------|------------------|------|----------|---------|-------------|--------|--------|----------|----------|----------------|--|----------|-----------------|------|---------------|--------|-----------|--------|------------|-----------|------------------------------------|---------------|---------|-----------------|-----------------|---|----------------------|---------------------------------|---|---------------------------|-------------------------------|---------------------------------------|------------------------|-----------------|----------------------------------|---|------------------------------------|---------------|---|-----------------------|----------------|--------------------|--|---|
| Company Comp | | (LT, RT, OR CL)' | TRUCTURE N | TOP ELEVATION | INVERT ELEVATION | F. | OPE | | (RC | | | |) | | | | C.S. I | PIPE | | | | | | | | | | | | RACTOR | RACTOR | | STD. 838.01 838.11 OR STD. 838.80 (UNLESS NOTED | QUANTITIES FOR DRAINAG | STRUCTURES *TOTAL F FOR P. | QUANTITY SHALL BE 'A' + (1.3 X COL.'B | 10.02 | | CONCRETE TRANSITIONAL SECTION | 7.2 GRATES STD. 840.29 | | | | , C.Y. STD. 840.71 | ⊢ | | C.B. N.D.I. D.I. G.D.I. G.D.I.(N.S.) | CATCH BASIN NARROW DROP INLET DROP INLET GRATED DROP INLET (NARROW SLOT) |
| The color The | THICKNESS | LOCAT | ОТ | | | | 12" | 15" 1 | 18" 24" | 30" 36 | 6" 42" | L | USE | DO NOT USE PVC | | | | | + | \Box | | 5" 18' | 3" 24" 30" | 36" 42" 4 | 8" 12" 1 | 15" 18" | 24" 30" | 36" 42 | 2" 48" | R.C. PIPE (CLASS V) RC PIPE CULVERTS, CC | RC PIPE CULVERTS, CC | SIDE DRAIN PIPE SIDE DRAIN PIPE | CU. YARDS | EACH (0' THRU 5.0') | | AND ABOVE B | STD. 840.01 OR STD. 84 | YPE OF GRATE | JP INLET | CH BASIN D.I STD. 840.35 I. (N.S. FLAT) FRAME W | DRAINAGE PIPE ELBOY | CLEANOUT (EA) | | IC. & BRICK PIPE PLUG | OLLARS CL. "B" | E REMOVAL LIN. FT. | M.H. T.B.D.I. | MANHOLE TRAFFIC BEARING DROP INLET TRAFFIC BEARING |
| 1 17 18 18 18 18 18 18 | -L- 15+75 | | | 30.8 | 27.6 | 27 A | + | | \parallel | | | | | + | # | | \boxminus | | <u> </u> | + | | | | | + | 24 | | | + | * * | ** 4 | 15" (| | 1 BER | 5.0' | 10.0, | C.B. | F G | DRO | 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | 15" [| HE HE | | NO CO | NO CO | PIPE | R | REMARKS |
| Control Cont | -L- 15+50 | LT 0401 | 1 | | | | | | | | | | | | 世 | | | | | | | | | | | 24 | | | | | | | | 1 | | | | | | 1 1 | | | | | | | | |
| March Marc | _l _ 15+80 | | | | 27.4 | 27.1 | $-\Box$ | | | | | | | | + | | $+\overline{1}$ | | + | | | | | | + | 24 | | $+\overline{+}$ | | | $+\overline{+}$ | | | 1 | | | \perp | | + | 1 1 | | | | $-\overline{T}$ | | | | |
| A COUNTY | | 0404 | 4 0403 | JU.Y | 27.7 | 27.1 | | | | | | | | | 廿 | | 世 | | | | | | | | | 28 | | | | | | | | 1 | | | | | | | | | | | | | | |
| ## 15 | -L- 15+50 | | | 30.6 | 27.1 | 22.8 | | 30 | | | | | Y | | \blacksquare | | \square | | | | | | | | | | | | | | | | | 1 | | | | | | 1 1 | 2 | | | | | | 15" W/ 2 EL BOWS | |
| Color Colo | -L- 15+50 | | | | 27.1 | 22.0 | | 30 | | | | | ^ | | 世 | | 世 | | | | | | | | | | | | | | | | | | | | | | | | 2 | 1 | | | | | | D-1 |
| MATERIAL PROPERTY AND PROPERT | | | | 24.2 | | | | \perp | | | | | | | oxdappi | | | | | | | | | | | | | | | | 4 | 48 | | 4 | | | | | | 4 4 | | | | | | 48 | REMOVE EXIST. 15" F | RCP |
| March Marc | -L- 18+10 | | | 31.3 | 28.1 | 27.8 | | ++ | | | | | | | ++ | | ++ | | | | | | | | + | 24 | | ++ | | | ++ | | | 1 | | | + | | | 1 1 1 | | | | | | | | |
| 1 | -L- 18+10 | | | 31.3 | | | | | | | | | | | \Box | | \Box | | | | | | | | | | | | | | | | | 1 | | | | | | 1 1 | | | | | | | | |
| | -DRIVE1- 10+45 | | | | 27.8 | 20.6 | | 40 | | | | | X | X | ++ | | + | | | | | | | | | | | ++ | | | ++ | 88 | | | | | | | | | 2 | | | | | 44 | | RCP |
| | | LT | | | | | | | | | | | | 二 | ightharpoons | | 廿 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | ++ | | | | | ++ | | | + | | + | | ++ | | + | | + | | | | | - | + | - | | ++ | | | ++ | | | | | | + | | | ++ | ++ | | | | | | | |
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| | | | \Box | | | | | | | | | | | | # | | 丗 | | ightharpoonup | | | | | | | | | | | | | | | | | | 士 | | | | | | | | | | | |
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| SHEET TOTALS 100 | | | | | | | | | | | | | | | \prod | | # | | | | | | | | \prod | | | | | | | | | | | | | | | | | | | | | | | |
| | SHEET TOTALS | | ++ | | | | | 70 | | | | | | _ | +++ | | + | | - | | | | +++ | | | 100 | | + + | | | 4 | 48 88 | | 6 | | | + | | | 6 6 | 3 4 · | 1 | | | | 132 | | |

COMPUTED BY: <u>Tyler Bottoms</u> DATE: <u>8/24/21</u>
CHECKED BY: <u>Jinyoung Park</u> DATE: <u>9/20/21</u>

(12-17-19)

PROJECT NO. SHEET NO. B-4607 3G-1

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

SUMMARY OF GEOTEXTILE FOR PAVEMIENT STABILIZATION

| LINE | Station | Station | Location LT/RT/CL | Drain Type* UD/BD/SD | LF |
|------|---------|---------|----------------------|-------------------------|-----|
| | | | | | |
| | | | | | |
| | | | | | |
| | CONTIN | | | 200 | |
| | | | | | |
| | | | | TOTAL LF: | 200 |
| | | | | | |

SUIMIMIARY OF SUIBSUIRFACE DRAINAGE

*SD = Subsurface Drain

SUMMARY OF ROCK PLATING

| LINE | Beginning Slope (H:V) | Approx. Station | Ending Slope (H:V) | Approx. Station | Location LT/RT | Rock Plating Detail No. 1/2/3/4 | Riprap Class* 1/2/B | Rock Plating SY |
|------|-----------------------------|--------------------|--------------------------|--------------------|-------------------|--|---------------------------|-----------------------|
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | | |
| | | | | | | | TOTAL SY: | 0 |
| I | | | | | | | | |

^{*}Use Class 1, 2 or B riprap if riprap class is not shown for rock plating location.

SUIMMARY OF AGGREGATE SUIBGRADE/STABILIZATION

| LINE | Station | Station | Aggregate Type* ASU(1/2)/ AST | Aggregate Thickness INCHES [8" for ASU(2)] | Shallow Undercut CY | Class IV Subgrade Stabilization TONS | Geotextile for Soil Stabilization SY | Stabilizer Aggregate TONS | Class IV Aggregate Stabilization TONS |
|------|------------|---------|--|--|---------------------------|---|---|---------------------------------|--|
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| (| CONTINGENC | Y | | | | | | | |
| | | | | | | | | | |
| | | | TOTAL | CY/TONS/SY: | 0 | 0** | 0** | 0 | 0 |
| | | | | | | | | | |

^{*}ASU(1/2) = Aggregate Subgrade (Type 1 or 2)

SUMMARY OF REINFORCED SOIL SLOPES AND SLOPE EROSION CONTROL

| LINE | Beginning Slope/ RSS (H:V) | Approx. Station | Ending Slope/ RSS (H:V) | Approx. Station | Location LT/RT | Reinforced Soil Slope (RSS) SY | Geocells SY | Coir Fiber Mat SY | Matting for Erosion Control SY |
|------|-------------------------------------|--------------------|----------------------------------|--------------------|-------------------|---|----------------|-------------------------|---|
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | TOTAL SY: | 0 | 0 | 0* | 0** |
| | | | | | | | | | |

^{*}Total square yards of "Coir Fiber Mat" is only the estimated quantity for slopes steeper than 2:1 (H:V) and may only represent a portion of the coir fiber mat quantity shown in the Item Sheets of the Proposal.

SUMMARY OF PRE-SPLITTING OF ROCK

| LINE | Beginning Rock Cut Slope (H:V) | Approx. Station | Ending Rock Cut Slope (H:V) | Approx. Station | Location LT/RT | Pre-splitting of Rock SY |
|------|---|--------------------|--------------------------------------|--------------------|-------------------|--------------------------------|
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | TOTAL SY: | 0 |
| | | | | | | |

SUIMMARY OF SUIRCHARGES AND SUIRCHARGE WAITING PERIODS

| LINE | Station | Station | Surcharge Height FT | MONTHS |
|------|---------|---------|---------------------------|--------|
| | | | | |
| | | | | |
| | | | | |

SUMMARY OF HORIZONTAL DRAINS

| LINE | Approximate Station | Location LT/RT | Elevation Above or Below Grade (+/-) FT | Inclination Angle DEGREES | PVC Pipe Schedule 40/80 or NO PIPE | Horizontal Drain FT | Horizontal Drain W/O Pipe FT |
|------|------------------------|-------------------|---|---------------------------------|---|---------------------------|---------------------------------------|
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |
| | C | ONTINGENCY | - | • | | | |
| | | | | | | | |
| | | | | | TOTAL FT: | 0 | 0 |
| | | | | | | | |

SUMMARY OF EMBANKMENT WAITING PERIODS

| LINE | Station | Station | MONTHS |
|------|---------|---------|--------|
| | | | |
| | | | |
| | | | |

SUMMARY OF SETTLEMIENT GAUGES

| Gauga | LINE and Station | Offset | | | |
|--------------|------------------------|----------------|--------------------|--|--|
| Gauge No. | | Distance FT | Direction LT/RT | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |
| | TOTAL GAI | · | | | |
| | | | | | |

SUMMARY OF BRIDGE WAITING PERIODS

| Bridge Description | End Bent/ Bent No. | MONTHS |
|--------------------|-----------------------|--------|
| | | |
| | | |
| | | |

^{*}UD = Underdrain
*BD = Blind Drain

LINE Station Station Station Station Sy Class IV Subgrade Stabilization TONS

CONTINGENCY

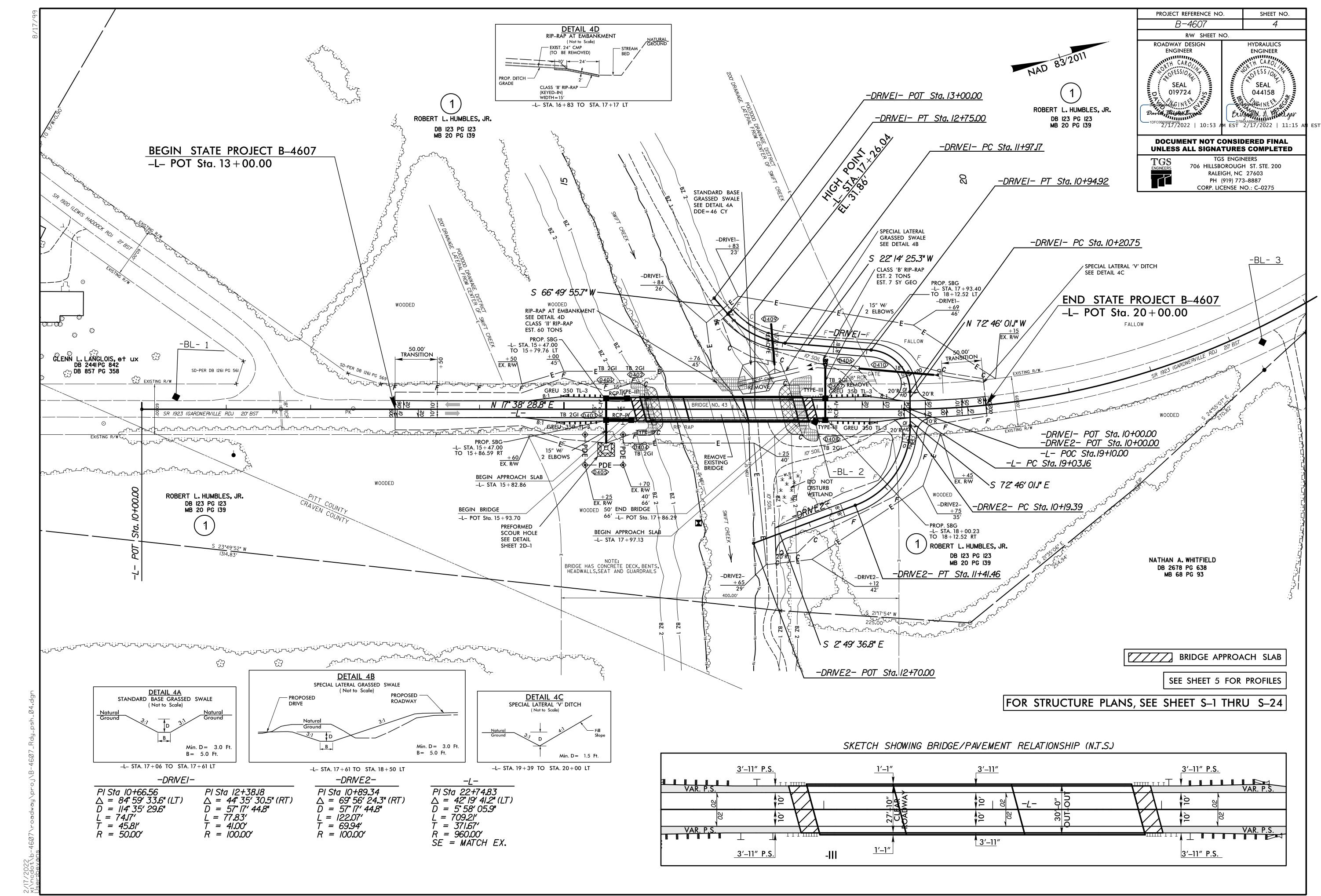
TOTAL SY/TONS: 0 0*

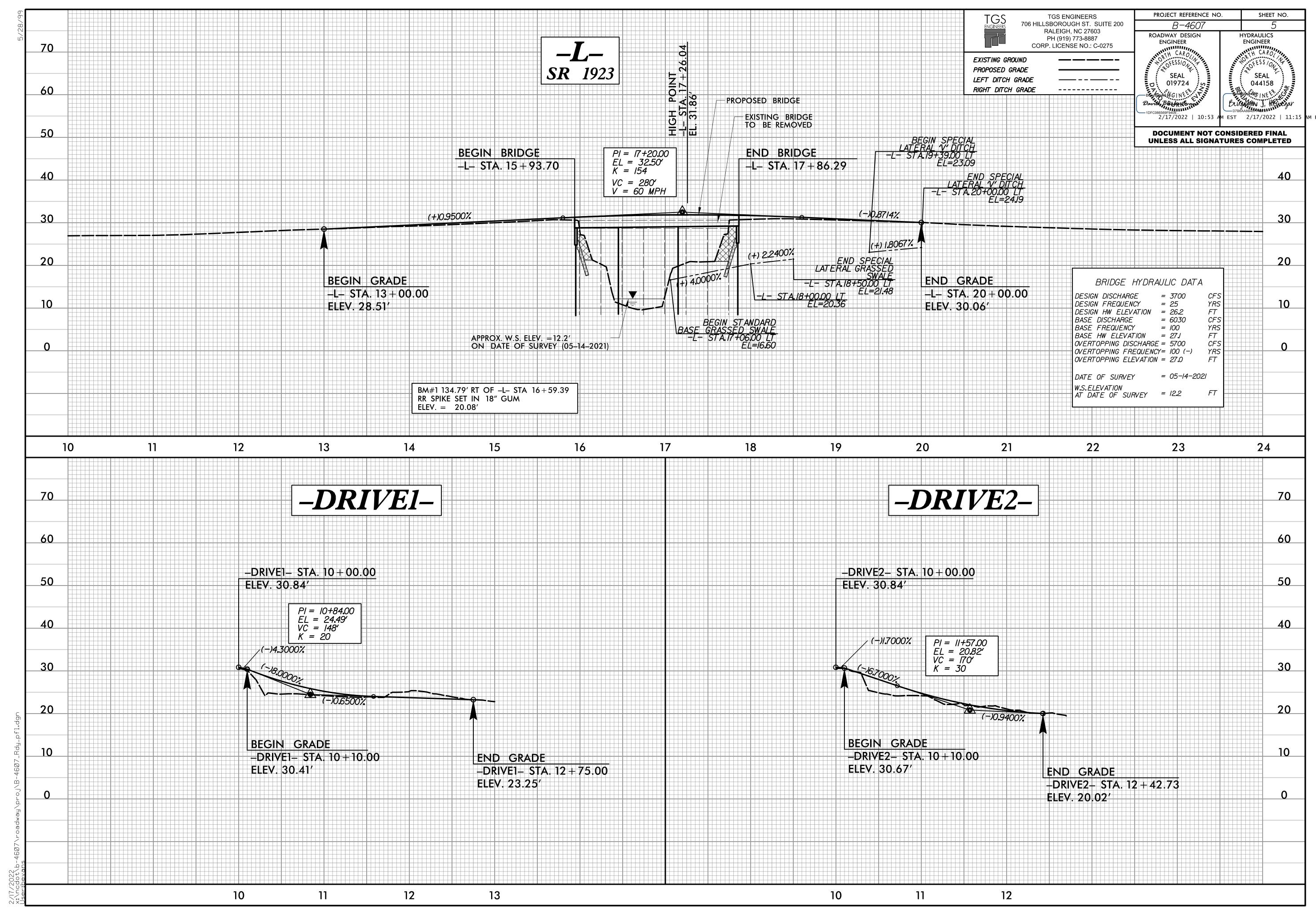
^{*}Total tons of "Class IV Subgrade Stabilization" is only the estimated quantity for pavement stabilization and may only represent a portion of the subgrade stabilization quantity shown in the Item Sheets of the Proposal.

^{*}AST = Aggregate Stabilization

^{**}Total tons of "Class IV Subgrade Stabilization" and total square yards of "Geotextile for Soil Stabilization" are only the estimated quantities for ASU(1/2)/AST and may only represent a portion of the subgrade stabilization and geotextile quantities shown in the Item Sheets of the Proposal.

^{**}Total square yards of "Matting for Erosion Control" is only the estimated quantity for RSS and may only represent a portion of the matting quantity shown in the Item Sheets of the Proposal.





PROJECT REFERENCE NO. SHEET NO.

B-4607 RW02C-1

Location and Surveys

DIVISION 2 LOCATION & SURVEYS

PROJECT SURVEYOR

CARO

SEAL

L-5082

SURVE

C. GREETING

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

I, James C. Green, III, PLS, certify that the Project Control was verified under my supervision from an actual GPS survey made under my supervision and the following information was used to perform the survey:

Class of survey: AA

Type of GPS field procedure: RTN

Dates of survey: SEPTEMBER 2019

Datum/Epoch: NAD 83 NA 2011

Published/Fixed-control use: RTN

Localized around: B4607-P5

Northing:597332.064

Easting: 2508291.881

Combined grid factor: 0.9998795945

Geoid model: G12NC

Units: US SURVEY FEET

I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:20,000 (Class AA) and Vertical accuracy to Class A. Field work was performed from Sept. 2019, and all coordinates are based on NAD 83/2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 15th day of November, 2021.

Docusigned by:

Aveen

Professional Land Surveyor L-5082





- 1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

PROJECT REFERENCE NO. SHEET NO.

B-4607 RW02C-2

Location and Surveys

DIVISION 2 LOCATION & SURVEYS

PROJECT SURVEYOR

CAROL

SEAL

L-5082

SURVE

C. GREETING

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Baseline

| BL | | | | | |
|----|-------|--------|-------------|--------------|-----------|
| | POINT | DESC. | NORTH | EAST | ELEVATION |
| | | | | | |
| 1 | | BL - 1 | 596733.9700 | 2507831.6360 | 26.02 |
| 2 | | BL - 2 | 597446.4240 | 2508089.0270 | 29.89 |
| 3 | | BL - 3 | 597980.1210 | 2508159.5900 | 27.83 |

Benchmark

Existing Alignments

| EL | | | | | | | | | |
|-------|------------|-------------|-----------------|--------|-----------------|-------------|--------|--------|--------|
| POINT | N | Е | BEARING | DIST | DELTA | D | L | T | R |
| POT | 596690.935 | 2507833.265 | | | | | | | |
| LINE | | | N 17°38′28.8" E | 9Ø3.16 | | | | | |
| PC | 597551.621 | 2508106.973 | | | | | | | |
| CURVE | | | N Ø3°31′21.8" W | 693.20 | 42°19′41.2"(LT) | Ø5°58′Ø5.9" | 7Ø9.21 | 371.67 | 960.00 |
| PT | 598243.507 | 2508064.380 | | | | | | | |
| LINE | | | N 24°41′12.5" W | 260.29 | | | | | |
| POT | 598480.004 | 2507955.670 | | | | | | | |

| EY | | | | | | | | | |
|-------------|------------|-------------|-----------------|--------|-----------------|-------------|-------|-------|--------|
| POINT | N | E | BEARING | DIST | DELTA | D | | T | R |
| POT LINE | 596730.461 | 2507642.441 | | | | | | | |
| LINE | | | N 53°Ø4′13.5" E | 156.35 | | | | | |
| PC | 596824.399 | 25Ø7767.419 | | | | | | | |
| CURVE PT | | | N 71°58′57.9" E | 78.33 | 37°49′28.8"(RT) | 47°25′1Ø.9" | 79.77 | 41.40 | 120.83 |
| | 596848.625 | 2507841.904 | | | | | | | |
| LINE POT | | | S 89°Ø6′17.7" E | 41.31 | | | | | |
| POT | 596847.980 | 2507883.207 | | | | | | | |

I, James C. Green, III, PLS, certify that the Project Control was verified under my supervision from an actual GPS survey made under my supervision and the following information was used to perform the survey:

Class of survey: *AA*Type of GPS field procedure: RTN
Dates of survey: SEPTEMBER 2019
Datum/Epoch:NAD 83 NA 2011
Published/Fixed-control use: RTN
Localized around: B4601-P5
Northing: 597332.064
Easting: 2508291.881
Combined grid factor: 0.9998795945
Geoid model: G12NC
Units: US SURVEY FEET

I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:20,000 (Class AA) and Vertical accuracy to Class A. Field work was performed from Sept. 2019 , and all coordinates are based on NAD 83/2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 15 day of November, 2021.

DocuSigned by:

Professional Land Surveyor L-5082

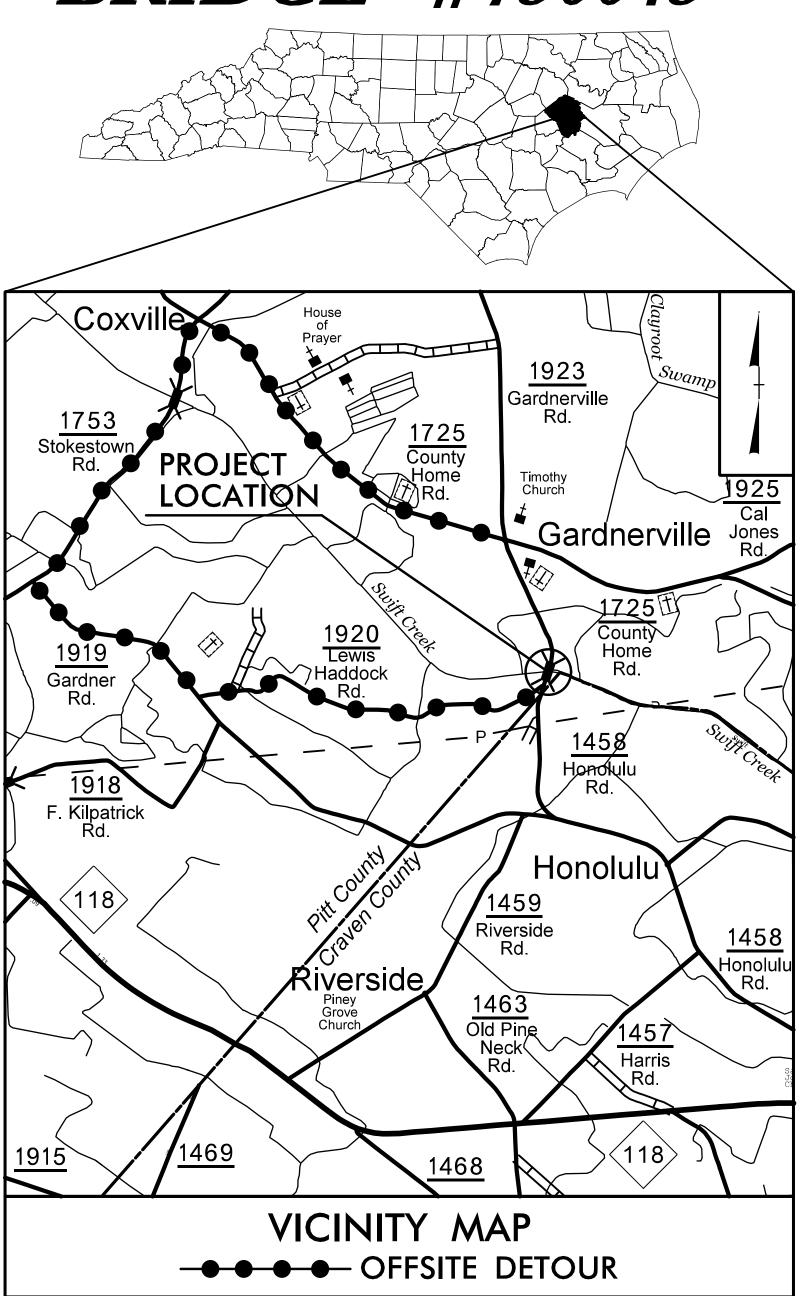


NOTES:

- 1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

mcrhodes1 AT LS-299767

PITT COUNTY BRIDGE #730043



NCDOT CONTACT INFORMATION: Phone: 252 439 2812 MICHAEL AMAN, PE DIVISION 2 BRIDGE PROGRAM MANAGER

PLAN PREPARED FOR N.C.D.O.T. BY:



TGS ENGINEERS TGS 706 HILLSBOROUGH ST. ENGINEERS SUITE 200 RALEIGH, NC 27603 PH (919) 773–8887 CORP. LICENSE NO. : C–0275

BURKE EVANS, PE DESIGN ENGINEER



INDEX OF SHEETS

SHEET NO.

TITLE

TMP - 1

TITLE SHEET, VICINITY MAP, INDEX OF SHEETS, LIST OF APPLICABLE ROADWAY STANDARD DRAWINGS,

AND LEGEND

PHASING, GENERAL NOTES, AND DETOUR TMP-2

GENERAL NOTES AND PHASING PLAN TMP-3

ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" -PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

TITLE

STD. NO.

| 1101.01 | WORK ZONE ADVANCE WARNING SIGNS |
|---------|--|
| 1101.03 | TEMPORARY ROAD CLOSURES |
| 1101.11 | TRAFFIC CONTROL DESIGN TABLES |
| 1110.01 | STATIONARY WORK ZONE SIGNS |
| 1110.02 | PORTABLE WORK ZONE SIGNS |
| 1145.01 | BARRICADES |
| 1165.01 | WORK VEHICLE LIGHTING SYSTEMS AND TMA DELINEATION |
| 1261.01 | GUARDRAIL AND BARRIER DELINEATORS - INSTALLATION SPACING |
| 1261.02 | GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING |
| 1262.01 | GUARDRAIL END DELINEATION |
| | |

NCDOT DIVISION 2 WILL PROVIDE AND MAINTAIN ALL TEMPORARY ROAD CLOSURE AND DETOUR SIGNING AND DEVICES.

LEGEND

<u>GENERAL</u>

DIRECTION OF TRAFFIC FLOW

DIRECTION OF PEDESTRIAN TRAFFIC FLOW

---- EXIST. PVMT.

PROPOSED PVMT.

TEMP. SHORING (LOCATION PURPOSES ONLY)

WORK AREA

PAVEMENT MARKINGS

——EXISTING LINES TEMPORARY LINES

TRAFFIC CONTROL DEVICES

BARRICADE (TYPE III)

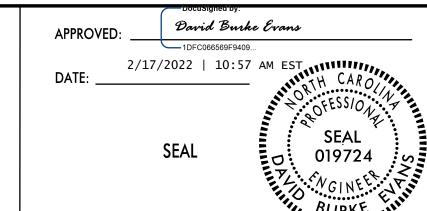
TEMPORARY SIGNING

PORTABLE SIGN

STATIONARY SIGN

STATIONARY OR PORTABLE SIGN

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



PROJECT ENGINEER

TMP-1

4607

GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

IN ORDER TO HAVE TIME TO ADEQUATELY REROUTE SCHOOL BUSSES, PITT COUNTY SCHOOLS WILL BE CONTACTED AT (252-916-0944) AT LEAST ONE MONTH PRIOR TO ROAD CLOSURE.

PITT COUNTY EMERGENCY SERVICES WILL BE CONTACTED AT (252-902-3652) AT LEAST ONE MONTH PRIOR TO ROAD CLOSURE TO MAKE THE NECESSARY TEMPORARY REASSIGNMENTS TO PRIMARY RESPONSE UNITS.

SIGNING

B) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

PAVEMENT MARKINGS AND MARKERS

C) NCDOT DIVISION 2 WILL INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE AS FOLLOWS:

MARKERS ROAD NAME MARKING SR 1923 (GARDNERVILLE RD) THERMOPLASTIC RAISED

- D) PASSING ZONE WILL BE DETERMINED IN THE FIELD AND MUST BE APPROVED BY THE ENGINEER.
- E) NCDOT DIVISION 2 WILL INSTALL AND MAINTAIN THE PROJECT DETOUR AND TYPE III BARRICADES AT THE PROJECT LIMITS. CONTACT JIM EVANS AT 252-439-2829 TWO WEEKS PRIOR TO CLOSING THE ROAD FOR DETOUR INSTALLATION.

PHASING

- STEP 1: NCDOT DIVISION 2 WILL INSTALL ALL DETOUR ROUTE SIGNS USING RSD 1101.01 AND AS SHOWN.
- STEP 2: NCDOT DIVISION 2 WILL INSTALL TYPE III BARRICADES AND SIGNS, AND CLOSE SR 1923 (GARDNERVILLE RD) TO TRAFFIC USING RSD 1101.03 (SHEET 1 OF 9) AND AS SHOWN.

NCDOT DIVISION 2 WILL PLACE TRAFFIC ON DETOUR.

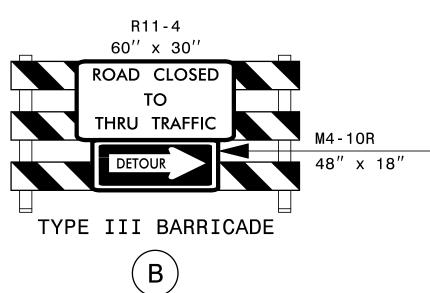
STEP 3: DEMOLISH AND REMOVE THE EXISTING BRIDGE OVER SWIFT CREEK.

CONSTRUCT THE NEW BRIDGE OVER SWIFT CREEK FROM -L- STA. 17+97.13 TO 17+86.29.

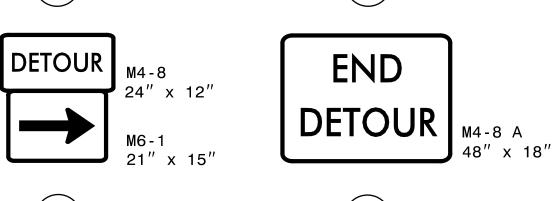
CONSTRUCT THE ROADWAY ON SR 1923 (GARDNERVILLE RD) FROM -L- STA. 13+00.00 TO -L- STA. 17+97.13 (BEGIN BRIDGE) AND FROM -L- STA. 17+86.29 (END BRIDGE) TO 20+00.00 UP TO AND INCLUDING THE FINAL LAYER OF SURFACE COURSE.

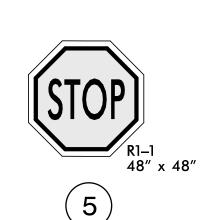
STEP 4: UPON COMPLETION OF BRIDGE, APPROACHES, AND ROADWAY, NCDOT DIVISION 2 WILL PLACE FINAL PAVEMENT MARKINGS AND MARKERS IN ACCORDANCE WITH RSD 1205.01, 1205.02, 1205.12, 1250.01, AND 1251.01. NCDOT DIVISION 2 WILL REMOVE BARRICADES, SIGNS, AND ALL OTHER TRAFFIC CONTROL DEVICES AND OPEN SR 1923 (GARDNERVILLE RD) TO TRAFFIC IN FINAL PATTERN.

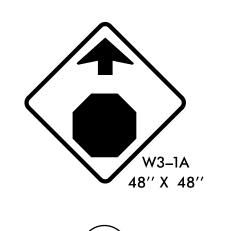


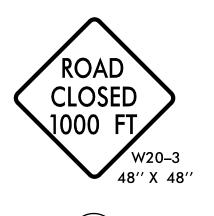


DETOUR | M4-8 DETOUR | M4-8 24" x 12" 24" x 12" (3)















(10)



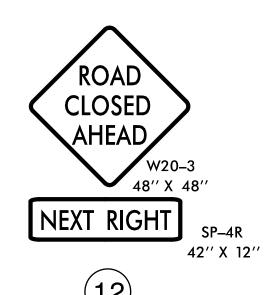
ROAD `

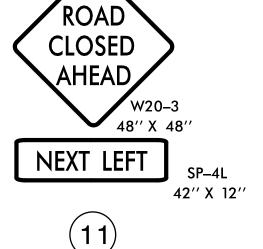
CLOSED

AHEAD /

W20-3

48" X 48"





(12)

* PLACE SIGN (3) ON RT SHOULDER FACING

ESTIMATED ADDITIONAL SIGNS REQUIRED PER RSD 1101.03:

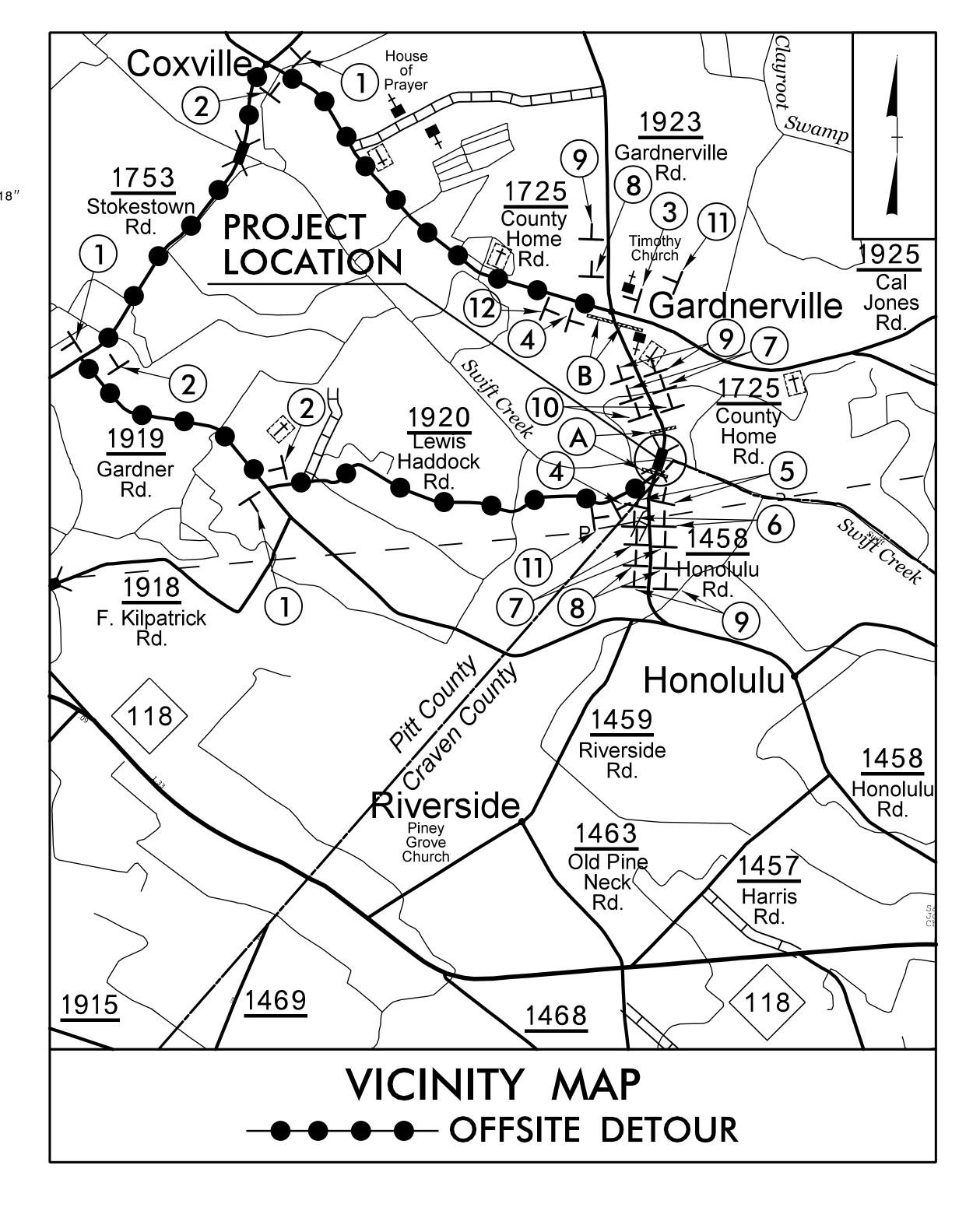
- W20-3 (18 EACH) - W20-2 (2 EACH)

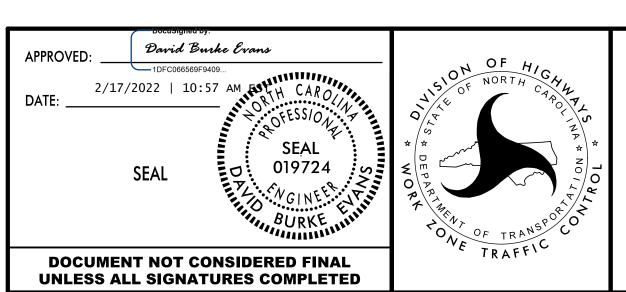
- SP-4 (6 EACH)

SEE RSD FOR SIGN PLACEMENT AND SIGN WORDING REQUIREMENTS.

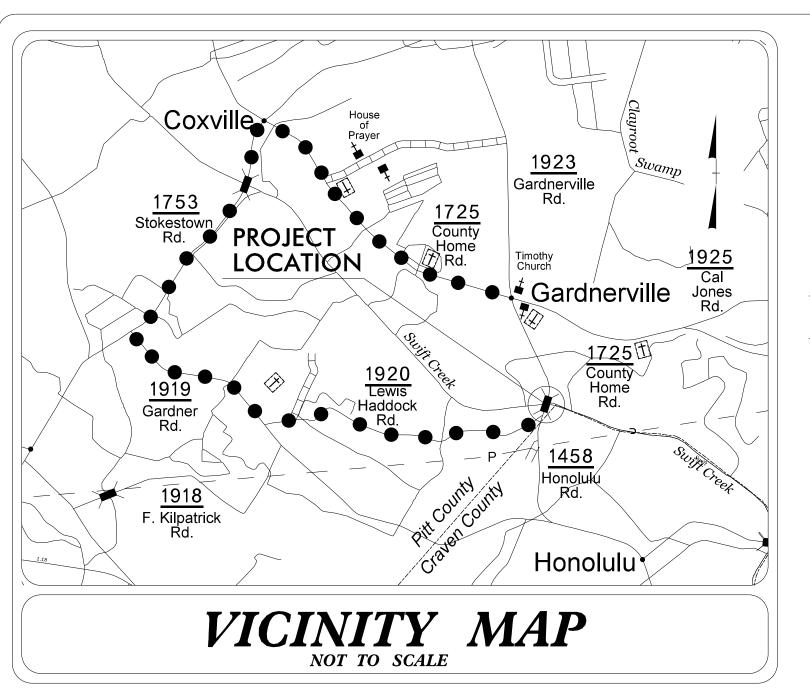
PROJ. REFERENCE NO. SHEET NO. B-4607 TMP-2

TGS ENGINEERS
706 HILLSBOROUGH ST.
NGINEERS SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO. : C-0275





GENERAL NOTES, PHASING, AND DETOUR

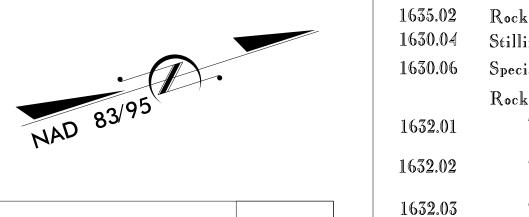


STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

FOR PROPOSED

LOCATION: BRIDGE NO. 43 OVER SWIFT CREEK ON SR 1923 (GARDNERVILLE RD.)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE



END STATE PROJECT B-4607 -L- STA. 20+00.00BEGIN BRIDGE -L- STA. 15 + 93.70 -DRIVE1-10 TO HONOLULU BRIDGE # 43 SR 1923 GARDNERVILLE RD. BEGIN STATE PROJECT B-4607 CRAVEN COUNTY END BRIDGE -DRIVE2--L- STA. 17 + 86.29

STATE PROJECT REFERENCE NO B=4607 DESCRIPTION

EROSION AND SEDIMENT CONTROL MEASURES

Temporary Silt Ditch

Temporary Silt Fence Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B. Temporary Rock Silt Check Type A. Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM) 1633.02 Temporary Rock Silt Check Type-B. Wattle / Coir Fiber Wattle Wattle / Coir Fiber Wattle with Polyacrylamide (PAM) Temporary Rock Sediment Dam Type-A. Temporary Rock Sediment Dam Type-B... Rock Pipe Inlet Sediment Trap Type-A Rock Pipe Inlet Sediment Trap Type-B. Stilling Basin Special Stilling Basin Rock Inlet Sediment Trap: Туре А Type B. Skimmer Basin Tiered Skimmer Basin Infiltration Basin

> ENVIRONMENTALLY SENSITIVE AREA(S) EXIST ON THIS PROJECT

THIS PROJECT CONTAINS

EROSION CONTROL PLANS FOR CLEARING AND

GRUBBING PHASE OF

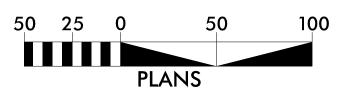
CONSTRUCTION.

Refer To E. C. Special Provisions for Special Considerations.

THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED STANDARDS.

GRAPHIC SCALE

PRELIMINARY PLANS



-L- STA. 13 + 00.00

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE APPLICABLE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE APRIL 1, 2019 AND ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENTAL QUALITY DIVISION OF WATER RESOURCES.



Prepared in the Office of:

TGS ENGINEERS

706 HILLSBOROUGH ST. - SUITE 200 RALEIGH, NC 27603

Designed by:

Ben Henegar, PE *3564* LEVEL III CERTIFICATION NO.

Roadway Standard Drawings

The following roadway <u>english</u> standards as appear in "Roadway Standard Drawings"— Roadway Design Unit — N. C. Department of Transportation — Raleigh, N. C., dated January 2018 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of these plans.

1604.01 Railroad Erosion Control Detail 1605.01 Temporary Silt Fence 1606.01 Special Sediment Control Fence 1607.01 Gravel Construction Entrance 1622.01 Temporary Berms and Slope Drains **1630.01** Riser Basin

1630.02 Silt Basin Type B 1630.03 Temporary Silt Ditch 1630.04 Stilling Basin 1630.05 Temporary Diversion

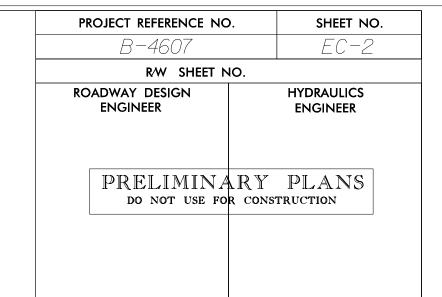
1630.06 Special Stilling Basin

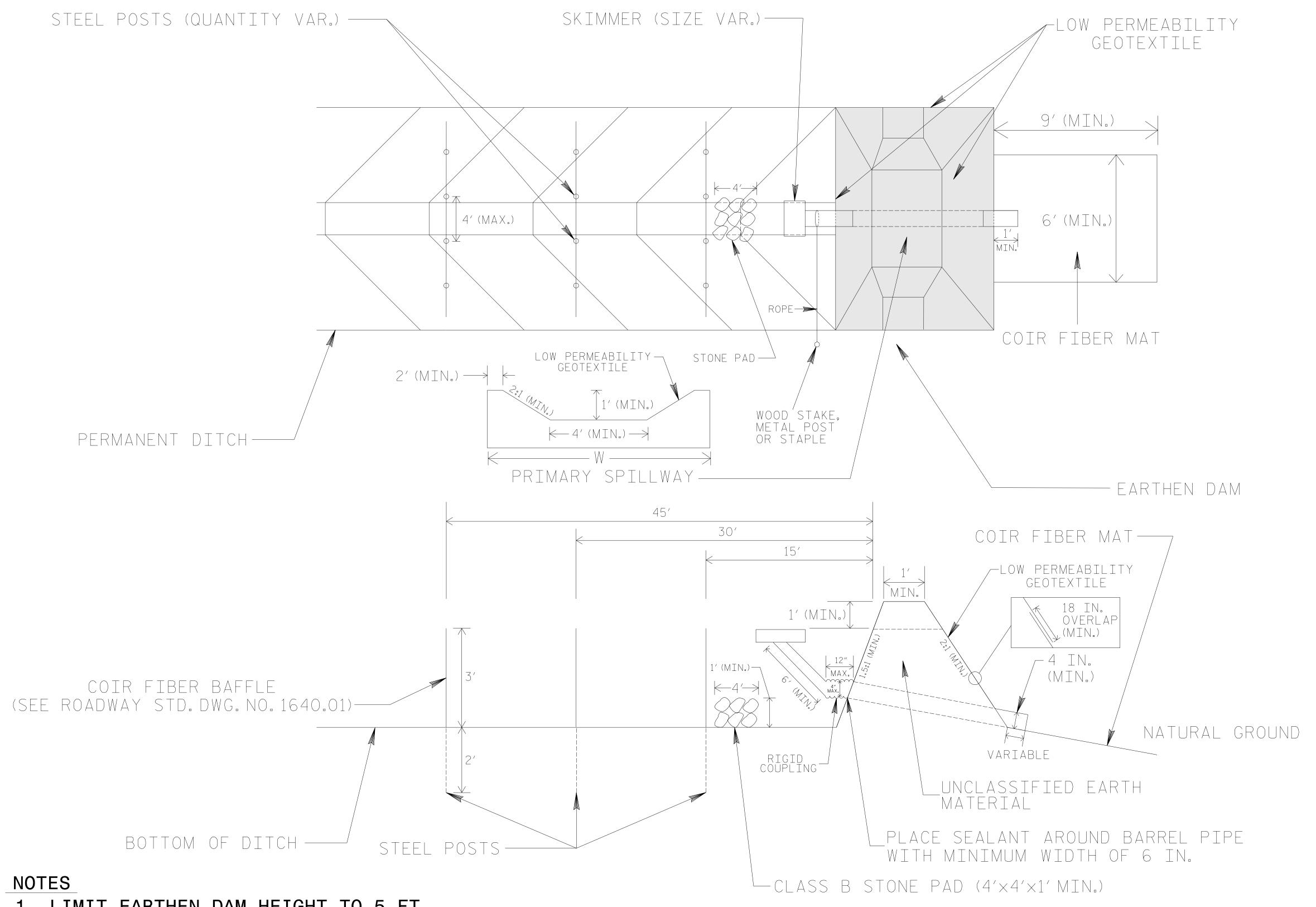
1631.01 Matting Installation

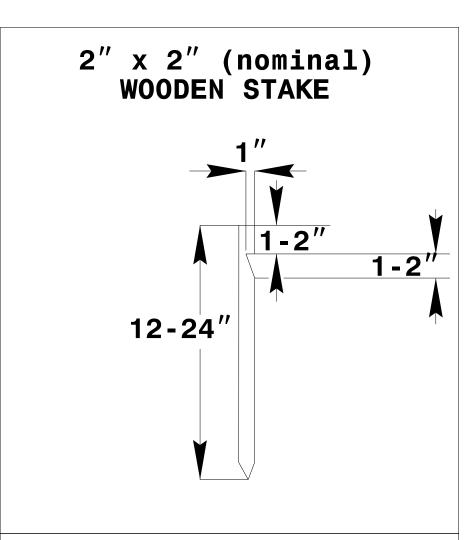
1632.01 Rock Inlet Sediment Trap Type A 1632.02 Rock Inlet Sediment Trap Type B 1632.03 Rock Inlet Sediment Trap Type C 1633.01 Temporary Rock Silt Check Type A 1633.02 Temporary Rock Silt Check Type B 1634.01 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type B
1640.01 Coir Fiber Baffle

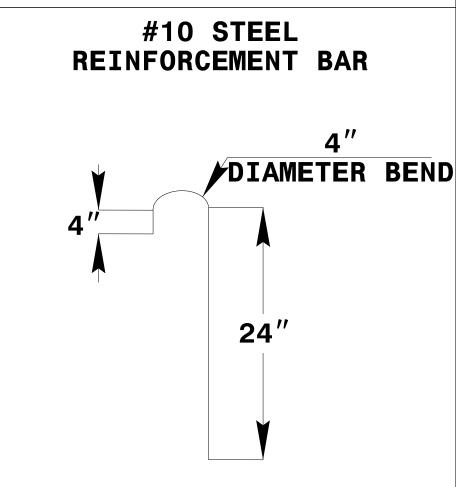
1645.01 Temporary Stream Crossing

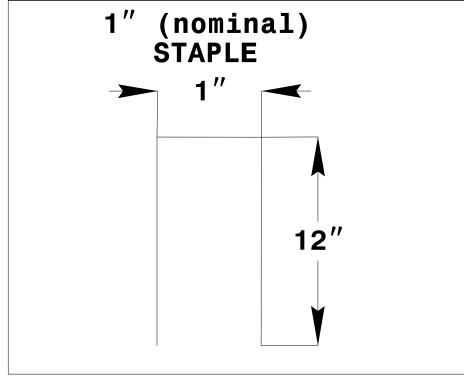
EARTHEN DAM WITH SKIMMER DETAIL (EAST)











COIR FIBER MAT ANCHOR OPTIONS

1. LIMIT EARTHEN DAM HEIGHT TO 5 FT. 2. DETERMINE PRIMARY SPILLWAY LENGTH (FT.) USING Q/O.8, WHERE Q IS FLOW RATE (CFS) INTO BASIN. 3. LOW PERMEABILITY GEOTEXTILE FOR PRIMARY SPILLWAY SHALL BE ONE CONTINUOUS PIECE OF MATERIAL OR OVERLAPPED 18 IN. (MIN.).

NOT TO SCALE

SILT FENCE COIR FIBER WATTLE BREAK DETAIL

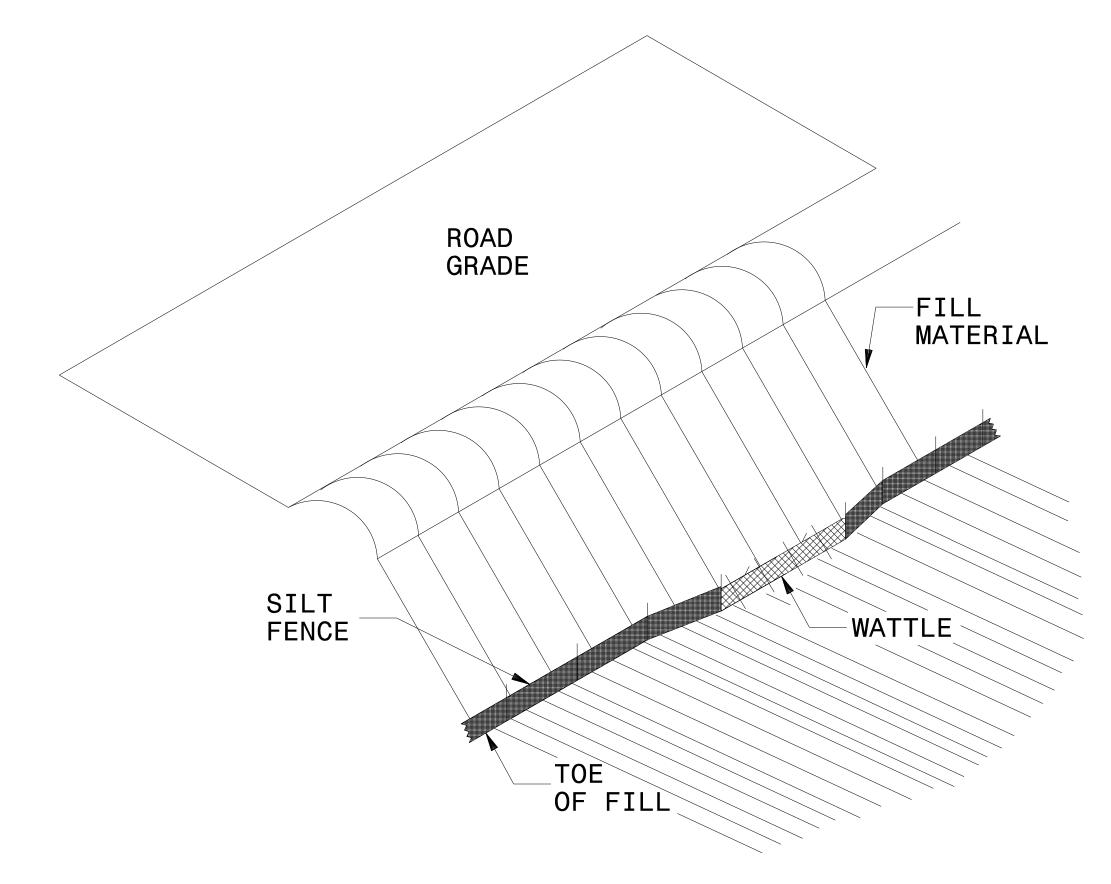
PROJECT REFERENCE NO.

B-4607

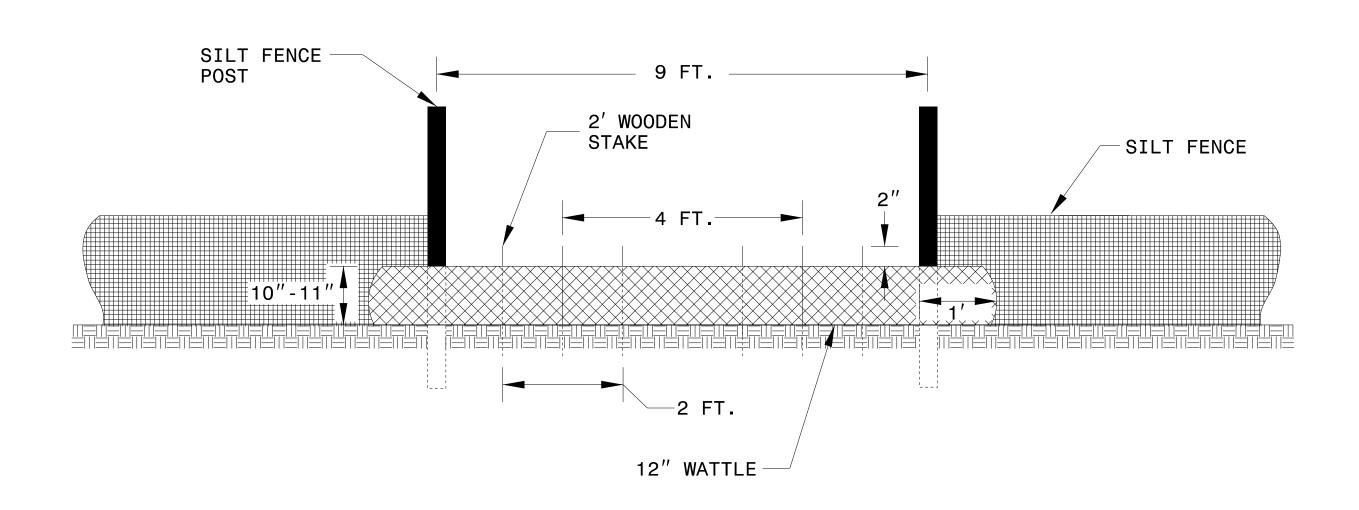
RW SHEET NO.

ROADWAY DESIGN
ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION



ISOMETRIC VIEW



VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

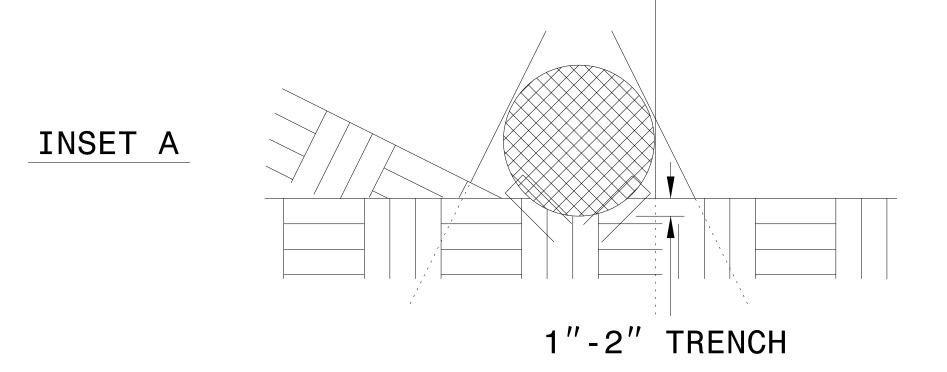
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

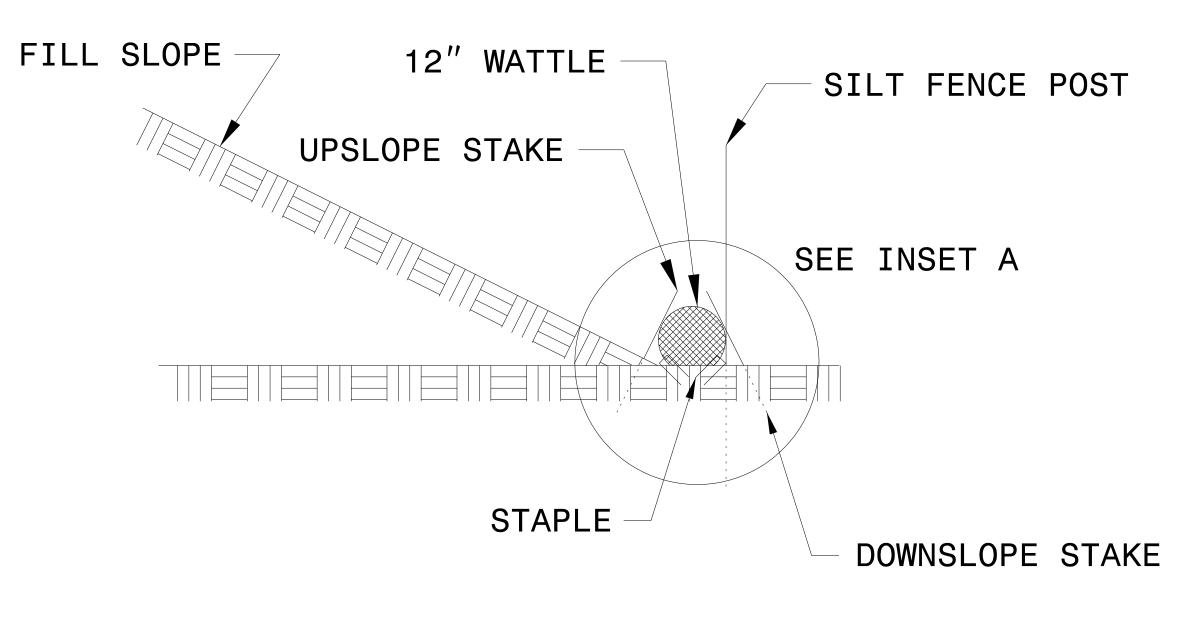
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.





SIDE VIEW

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

| PROJECT REFERENCE NO | PROJECT REFERENCE NO. | | | | | |
|-----------------------------|------------------------|--|--|--|--|--|
| B-4607 | B-4607 | | | | | |
| | | | | | | |
| ROADWAY DESIGN ENGINEER | HYDRAULICS ENGINEER | | | | | |
| PRELIMINA DO NOT USE FOR | _ | | | | | |
| | | | | | | |

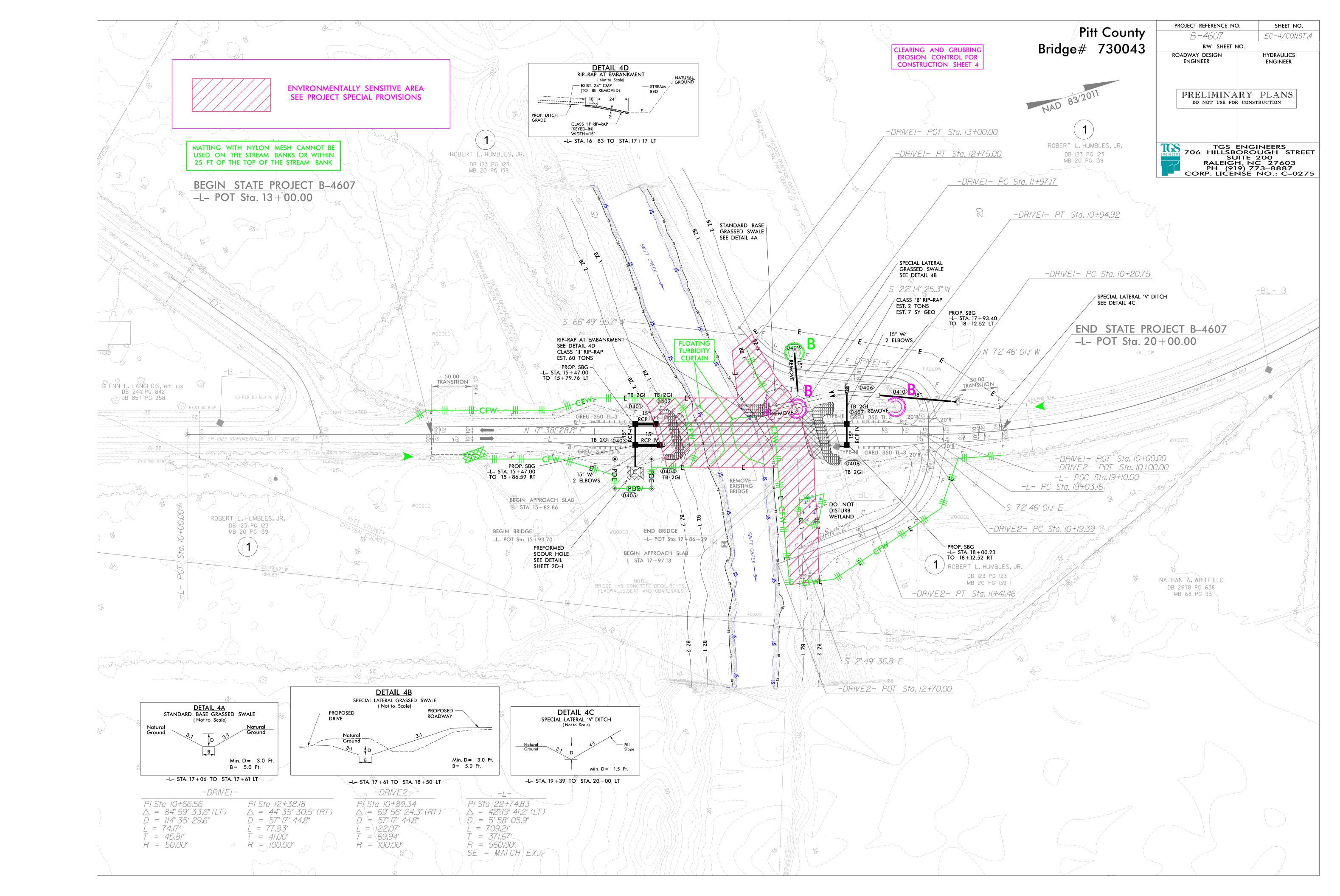
SOIL STABILIZATION TIMEFRAMES

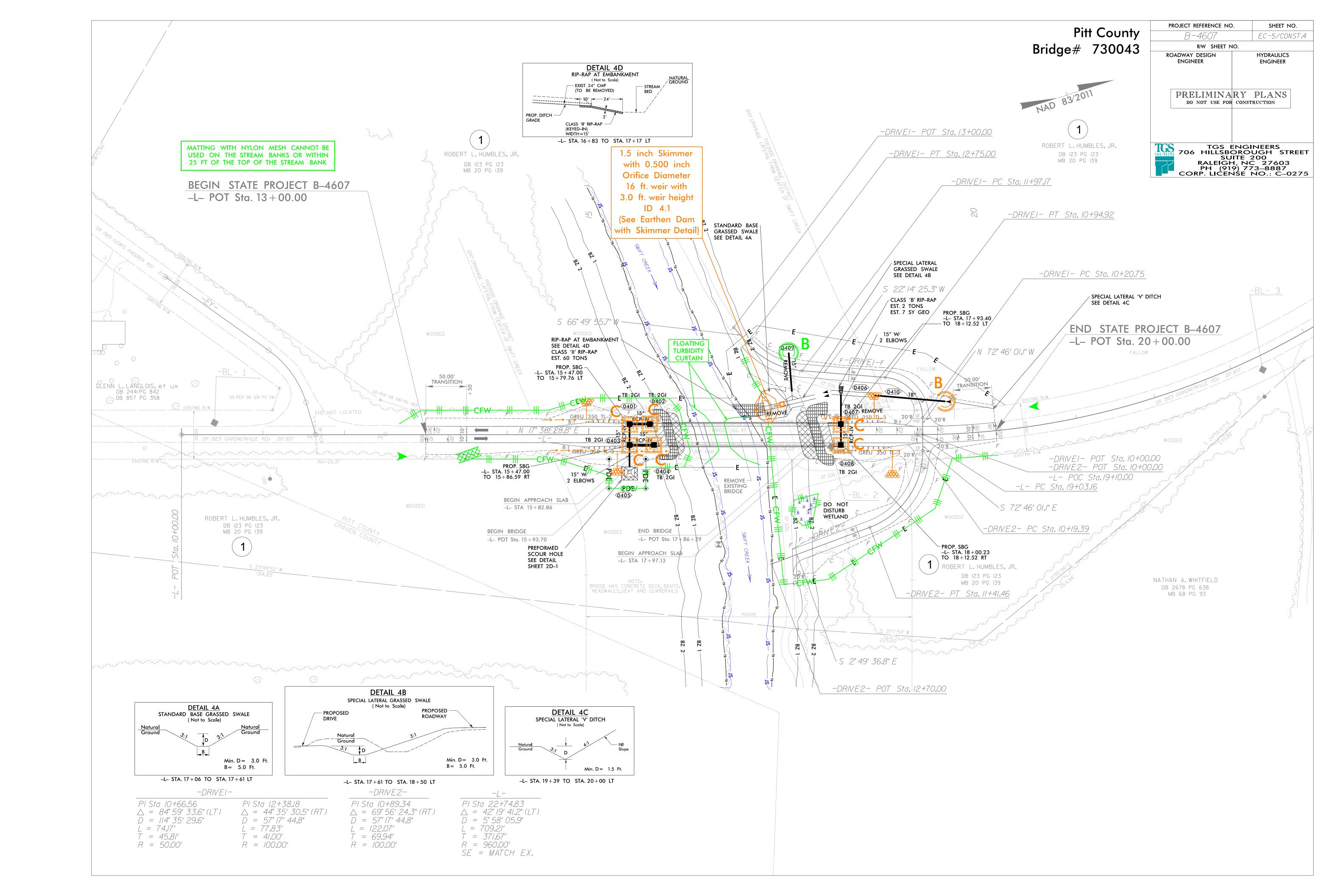
| SITE DESCRIPTION | STABILIZATION TIME | TIMEFRAME EXCEPTIONS |
|--|--------------------|---|
| PERIMETER DIKES, SWALES, DITCHES AND SLOPES | 7 DAYS | NONE |
| HIGH QUALITY WATER (HQW) ZONES | 7 DAYS | NONE |
| SLOPES STEEPER THAN 3:1 | 7 DAYS | IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED. |
| SLOPES 3:1 OR FLATTER | 14 DAYS | 7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH. |
| ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1 | 14 DAYS | NONE, EXCEPT FOR PERIMETERS AND HQW ZONES. |

SOIL STABILIZATION SUMMARY SHEET

MATTING FOR EROSION CONTROL

T0 CONST FROM SIDE L/NE ESTIMATE (SY) STAT/ON STAT/ON SHEET NO. 17+06 8 + 50 385 - L - DITCH 4 65 19+39 20+00 - L - DITCH 15+80 215 14+50 -L- SLOPE 4 275 15+85 RT 14+50 -L- SLOPE 4 9+00 290 17+93 4 -L- SLOPE 19+00 180 8+00 4 -L- SLOPE SUBTOTAL 1410 MISCELLANEOUS MATTING TO BE INSTALLED AS DIRECTED BY THE ENGINEER 1555 TOTAL 2965 2965 SAY



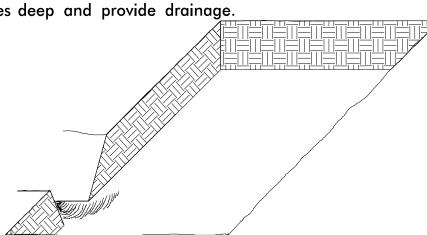


PLANTING DETAILS

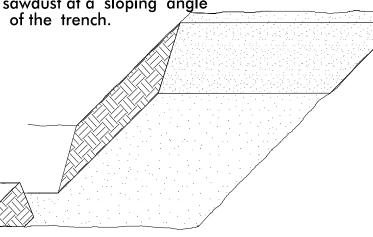
SEEDLING / LINER BAREROOT PLANTING DETAIL

HEALING IN

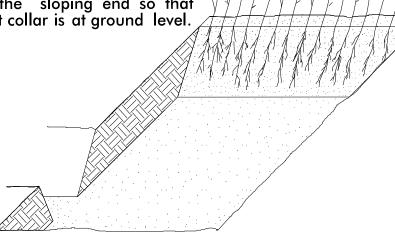
- Locate a healing-in site in a shady, well protected area.
- Excavate a flat bottom trench
 inches deep and provide drainage.



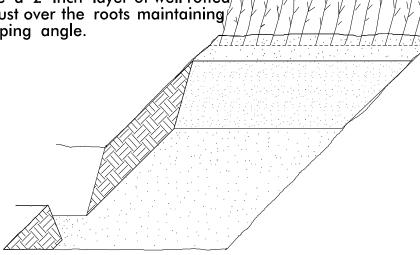
3. Backfill the trench with 2 inches well rotted sawdust. Place a 2 inch layer of well rotted sawdust at a sloping angle at one end of the trench.



4. Place a single layer of plants against the sloping end so that the root collar is at ground level.

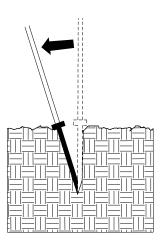


5. Place a 2 inch layer of well rotted sawdust over the roots maintaining a sloping angle.

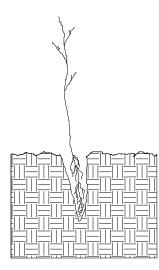


Repeat layers of plants and sawdust as necessary and water thoroughly.

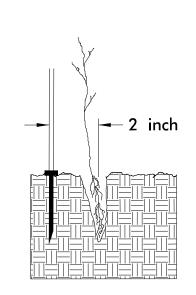
DIBBLE PLANTING METHOD USING THE KBC PLANTING BAR



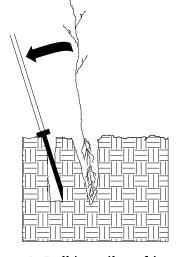
Insert planting bar as shown and pull handle toward planter.



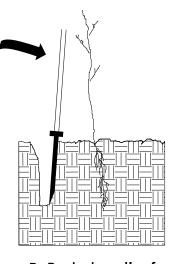
Remove planting bar and place seedling at correct depth.



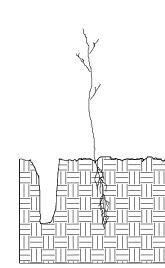
3. Insert planting bar 2 inches toward planter from seedling.



Pull handle of bar toward planter, firming soil at bottom.



Push handle forward firming soil at top.



Leave compaction hole open. Water thoroughly.

PLANTING NOTES:

PLANTING BAG
During planting, seedlings
shall be kept in a moist
canvas bag or similar container to prevent the root systems from drying.



KBC PLANTING BAR
Planting bar shall have a
blade with a triangular
cross section, and shall
be 12 inches long,
4 inches wide and
1 inch thick at center.

ROOT PRUNING
All seedlings shall be root
pruned, if necessary, so that
no roots extend more than
10 inches below the
root collar.



| STATE | STATE | SHEET NO. | TOTAL SHEETS | | |
|-------|-----------|-----------------|-----------------|---------|-------|
| N.C. | | RF-1 | | | |
| STATE | PROJ. NO. | F. A. PROJ. NO. | | DESCRIF | PTION |

REFORESTATION

TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

REFORESTATION

MIXTURE, TYPE, SIZE, AND FURNISH SHALL CONFORM TO THE FOLLOWING:

| TULIP POPLAR | 12 in - 18 in BR |
|--------------|-------------------------|
| SYCAMORE | 12 in - 18 in BR |
| RIVER BIRCH | 12 in - 18 in BR |
| BLACK GUM | 12 in - 18 in BR |
| | SYCAMORE RIVER BIRCH |

REFORESTATION DETAIL SHEET

N.C.D.O.T. – ROADSIDE ENVIRONMENTAL UNIT

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

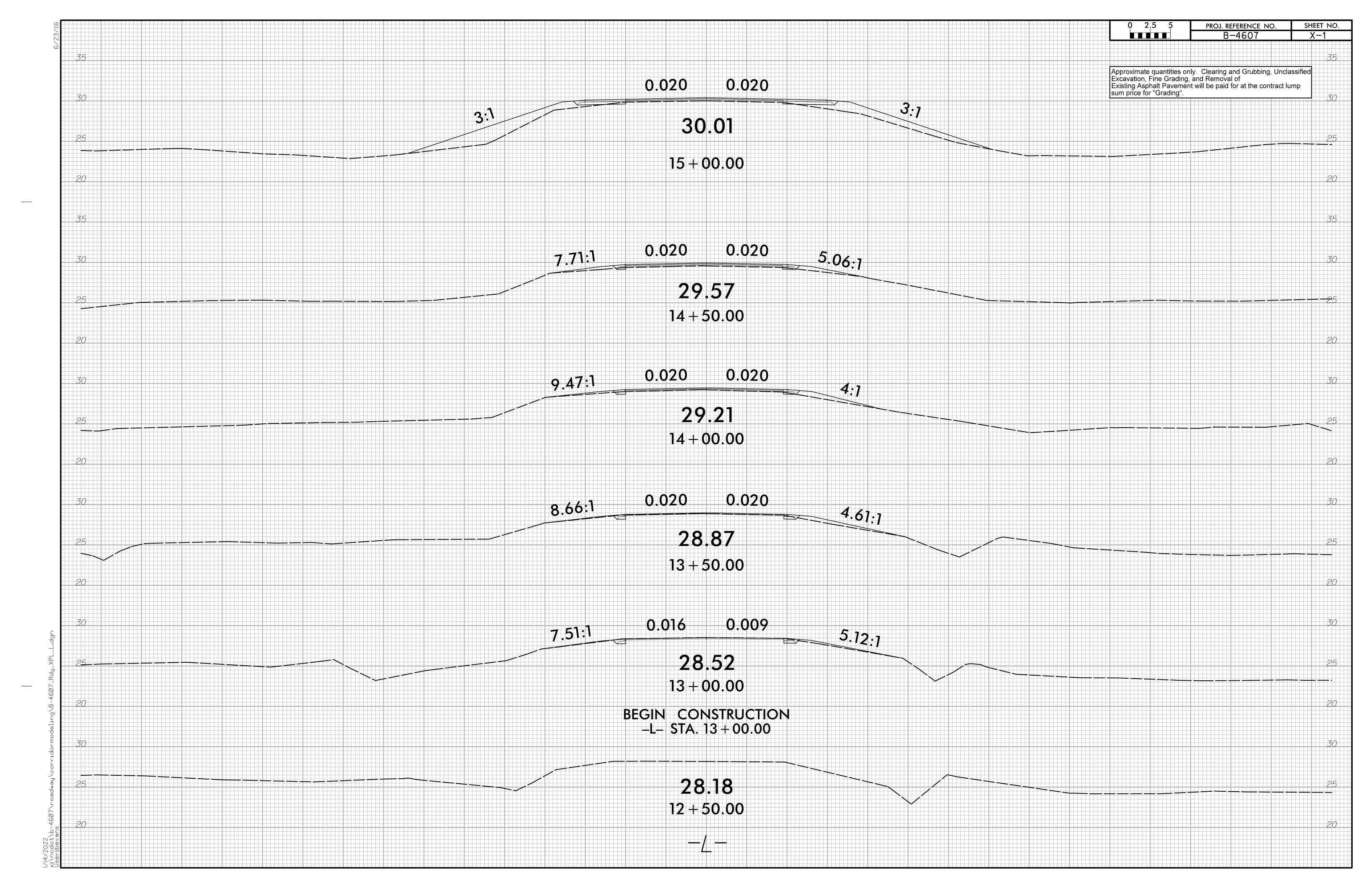
PROJ. REFERENCE NO. SHEET NO.

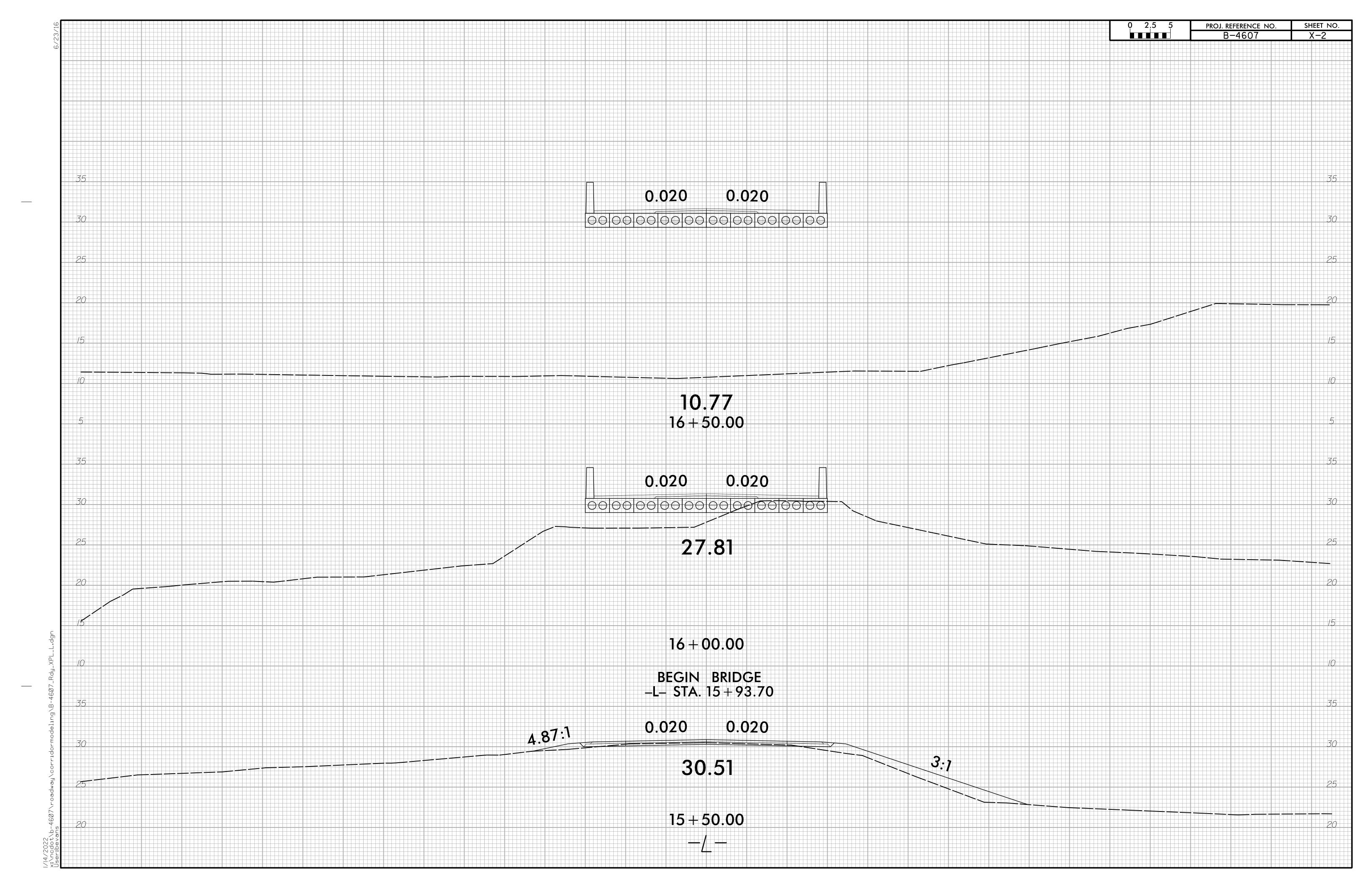
B-4607 X-1A

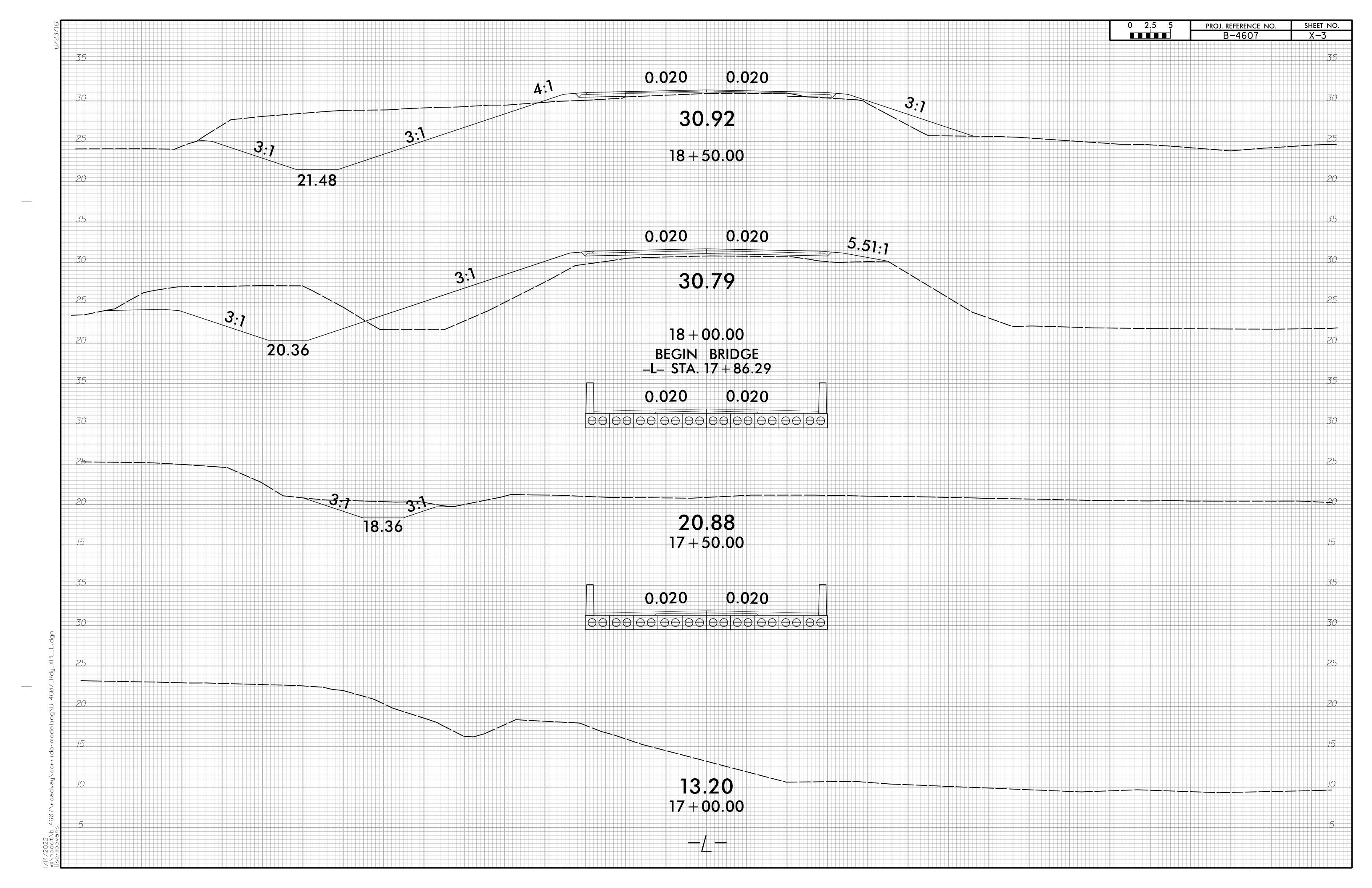
NOTE: EMBANKMENT COLUMN DOES NOT INCLUDE BACKFILL FOR UNDERCUT

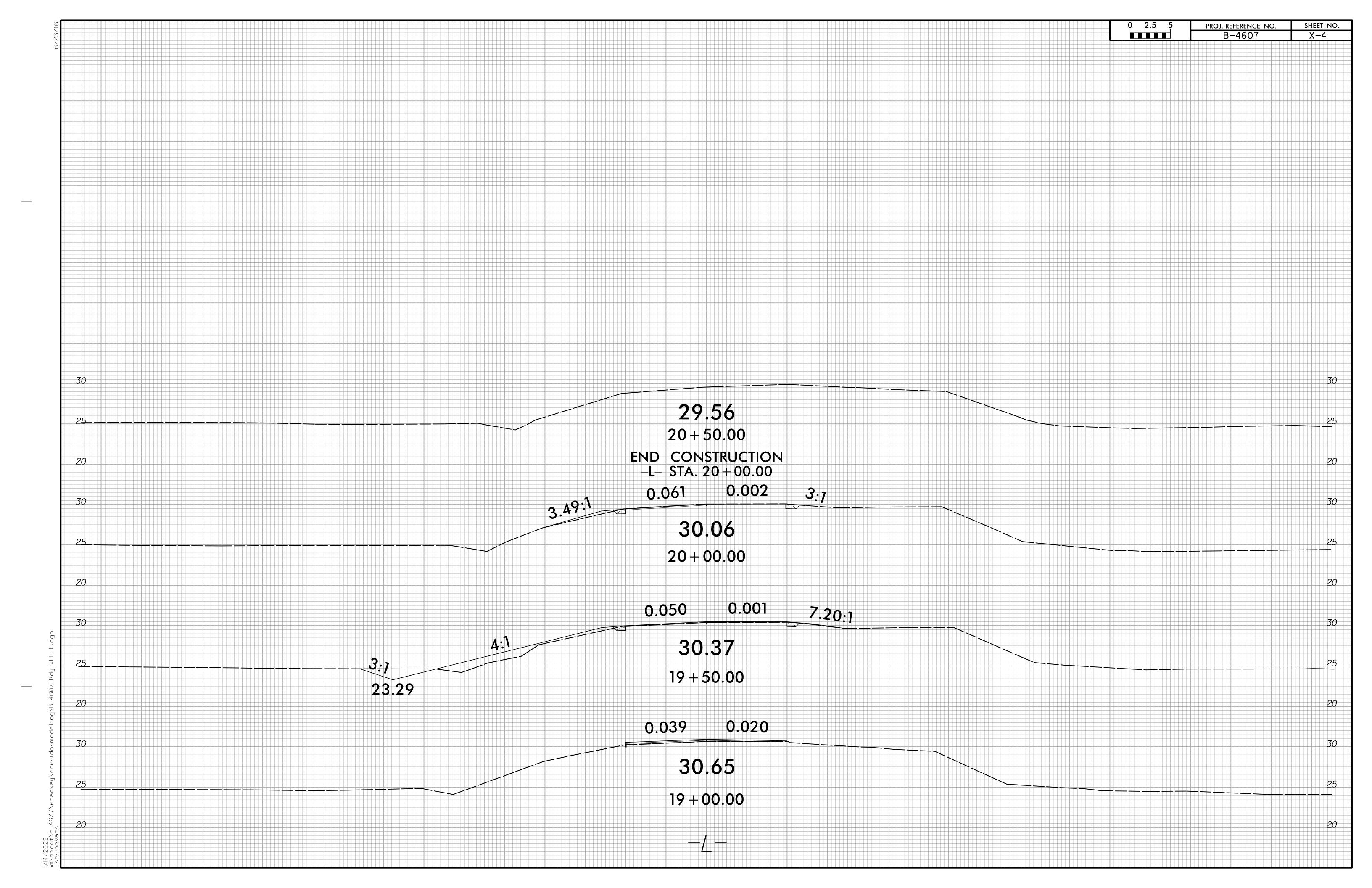
CROSS-SECTION SUMMARY

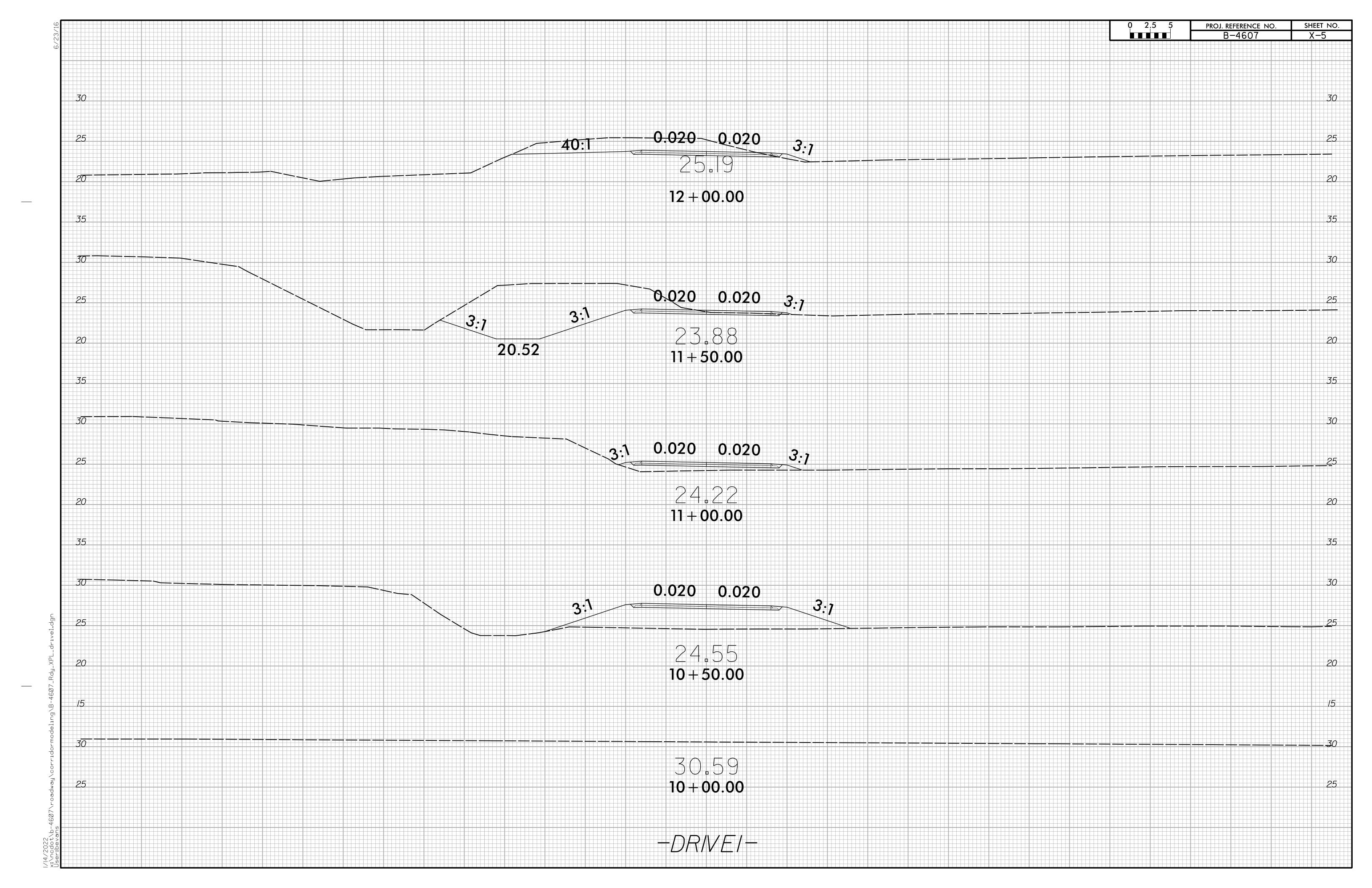
| NOTE: EMBANKI | MENT COLUMN DOES | S NOT INCLUDE E | ACKFILL FOR UNDERCUT CRUSS-SECTION SUMMARY | | | | |
|---------------|------------------|-----------------|---|--|----------------------|-----------------------|-----|
| Station | Uncl. Exc. | Embt | | | | | |
| | | | Appr | oximate quantities o | nlv. Unclassified ex | cavation. fine gradir | na. |
| L | (cu. yd.) | (cu. yd.) | clear | Approximate quantities only. Unclassified excavation, fine grading clearing and grubbing and removal of existing pavement will be paid | | | |
| 13+00.00 | 0 | 0 | at the | e lump sum price for | "Grading". | | |
| 13+50.00 | 3 | 6 | | | | | |
| 14+00.00 | 2 | 9 | | | | | |
| 14+50.00 | 1 | 9 | | | | | |
| 15+00.00 | 1 | 42 | | | | | |
| 15+50.00 | 4 | 66 | | | | | |
| 15+93.70 | 7 | 48 | | | | | |
| | | - | | | | | |
| Station | Uncl. Exc. | Embt | | | | | |
| | | | | | | | |
| L | (cu. yd.) | (cu. yd.) | | | | | |
| 17+86.29 | 0 | 0 | | | | | |
| 18+00.00 | 64 | 49 | | | | | |
| 18+50.00 | 278 | 108 | | | | | |
| 19+00.00 | 161 | 19 | | | | | |
| 19+50.00 | 7 | 13 | | | | | |
| 20+00.00 | 9 | 14 | | | | | |
| 2 20.00 | | • • • | | | | | |
| Station | Uncl. Exc. | Embt | | | | | |
| | | | | | | | |
| Drive1 | (cu. yd.) | (cu. yd.) | | | | | |
| 10+10.00 | 0 | 0 | | | | | |
| 10+50.00 | 0 | 109 | | | | | |
| 11+00.00 | 0 | 79 | | | | | |
| 11+50.00 | 124 | 11 | | | | | |
| 12+00.00 | 168 | 1 | | | | | |
| 12+50.00 | 65 | 1 | | | | | |
| 12+75.00 | 14 | 0 | | | | | |
| | | | | | | | |
| Station | Uncl. Exc. | Embt | | | | | |
| Drive2 | (cu. yd.) | (cu. yd.) | | | | | |
| 10+10.00 | 0 | 0 | | | | | |
| 10+50.00 | 0 | 116 | | | | | |
| 11+00.00 | 0 | 79 | | | | | |
| 11+50.00 | 12 | 8 | | | | | |
| 12+00.00 | 37 | 2 | | | | | |
| 12+42.73 | 29 | 0 | | | | | |
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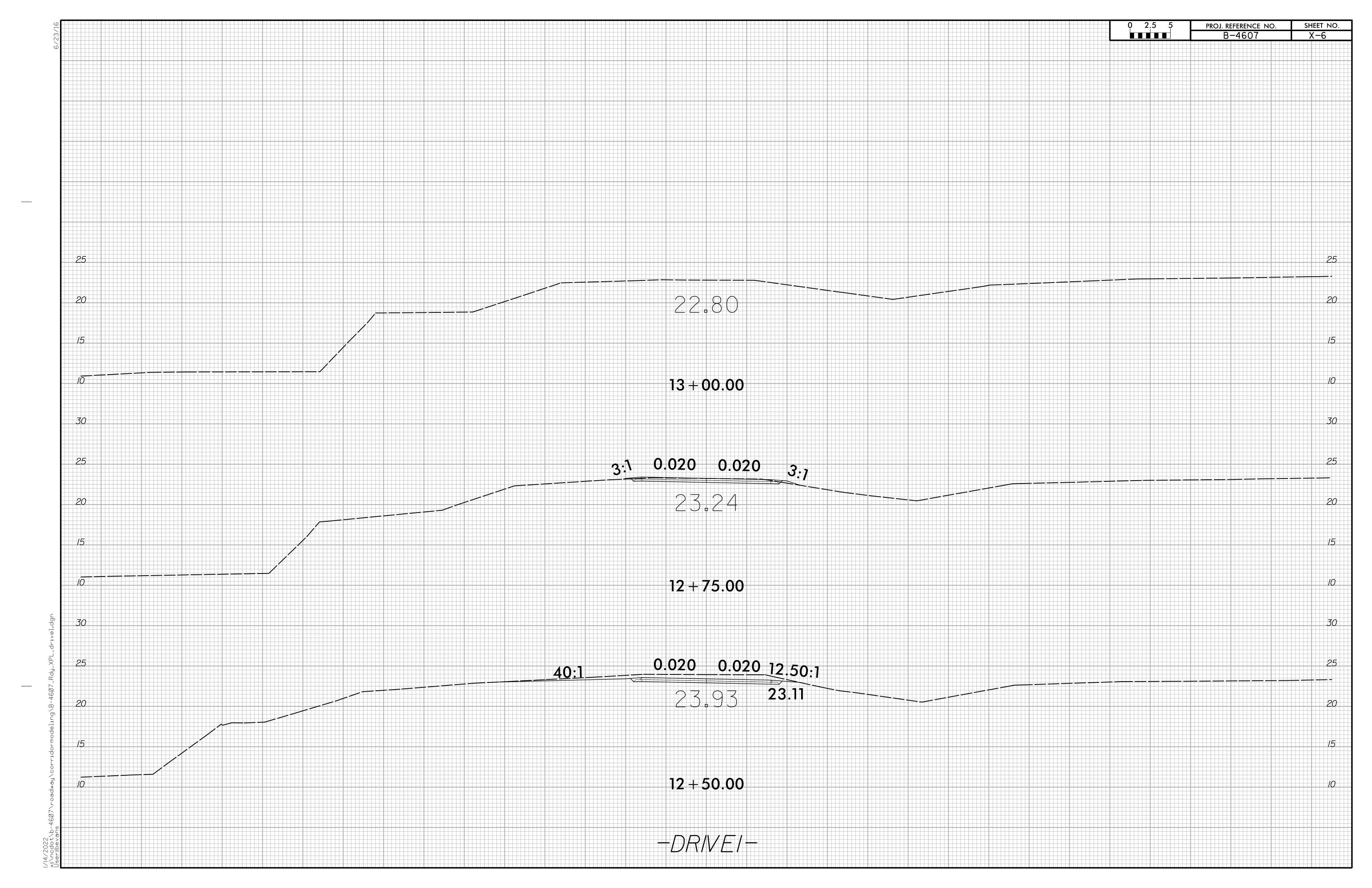


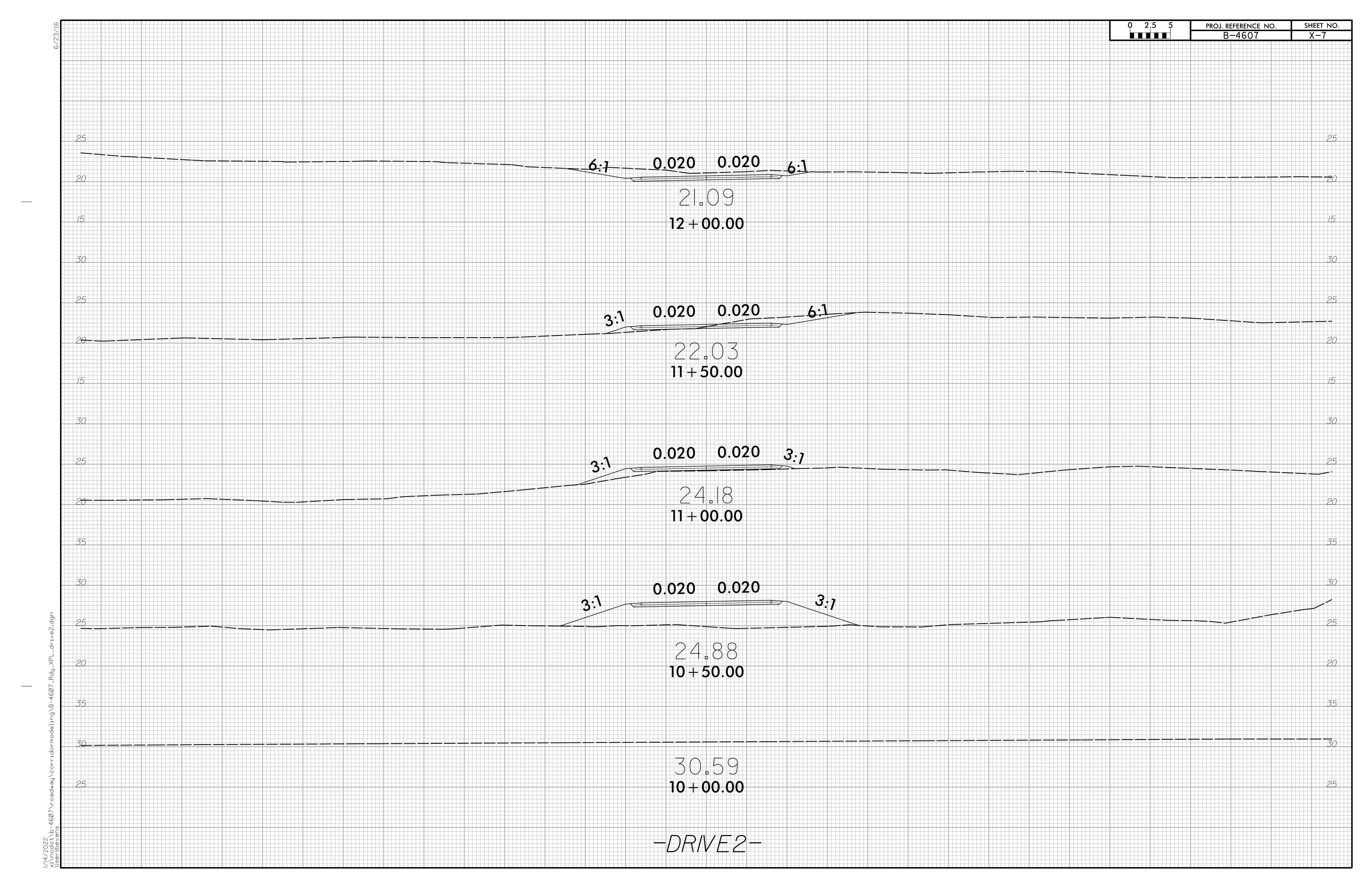












09 M Gardnerville Jones Rd. 1458 Honolulu 1918 F. Kilpatrick Rd. Honolulu 1458 Honolulu Rd. Riverside ± <u>1469</u> VICINITY MAP → ◆ ◆ ◆ OFFSITE DETOUR TO HONOLULU BEGIN STATE PROJECT B-4607 -L-STA. 13 + 00.00

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

PITT COUNTY

LOCATION: REPLACE BRIDGE NO. 43 ON SR 1923 OVER SWIFT CREEK

TYPE OF WORK: GRADING, DRAINAGE, STRUCTURE AND PAVING

END STATE PROJECT B-4607

Prepared For:

DIVISION OF HIGHWAYS

1037 W. H. Smith Blvd, Greenville NC, 27834

TGS ENGINEERS

706 HILLSBOROUGH ST.

SUITE 200

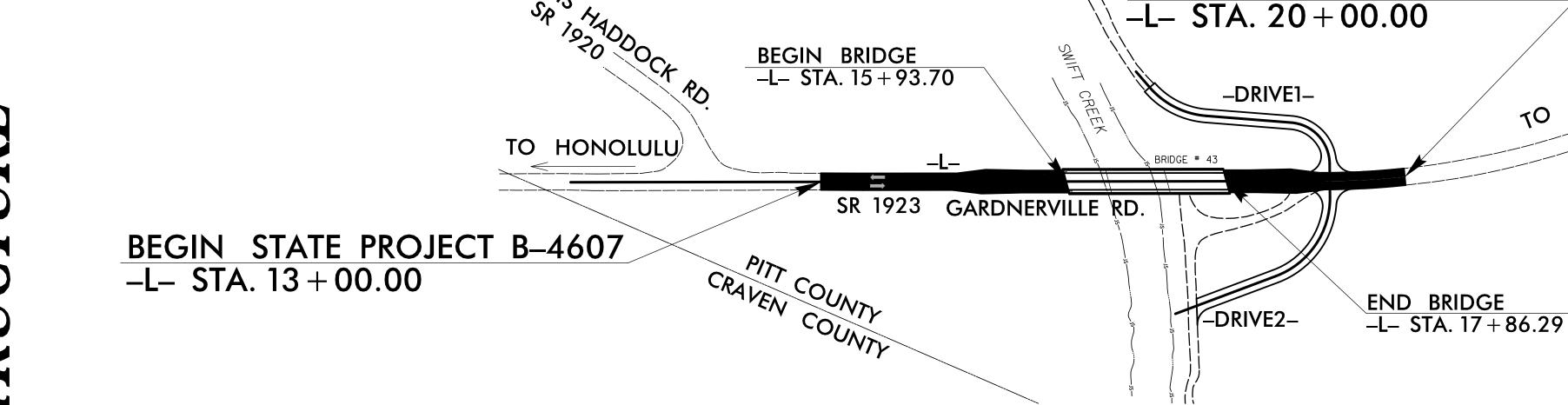
RALEIGH, NC 27603

LETTING DATE:

APRIL 27, 2022

| STATE | STATE | PROJECT REFERENCE NO. | NO. | SHEETS | | |
|-------|-------------|-----------------------|----------|------------|--|--|
| N.C. | | B-4607 | | | | |
| STAT | E PROJ. NO. | F. A. PROJ. NO. | DESCRIPT | ION | | |
| 3843 | 32.1.FD2 | BRZ-1923(11) | PE | | | |
| 3843 | 32.2.2 | BRZ-1923(11) | ROW/ U | ROW/ UTIL. | | |
| 3843 | 32.3.3 | CONSTR. | | | | |
| | | | | | | |
| | | | | | | |
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ONTRACT: DI

DESIGN DATA

ADT 2019 = 650 ADT 2040 = 1060

T = 6% V = 55 MPH

FUNCT CLASS=RURAL LOCAL

SUB-REGIONAL TIER DESIGN STANDARDS

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4607 = 0.097 mile

LENGTH STRUCTURES TIP PROJECT B-4607 = 0.036 mile

TOTAL LENGTH TIP PROJECT B-4607 = 0.133 mile

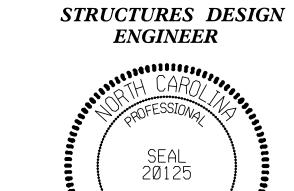
e 2018 STANDARD SPECIFICATIONS

MARC CHEEK, PE STRUCTURES DESIGN ENGINEER

PH (919) 733–8887

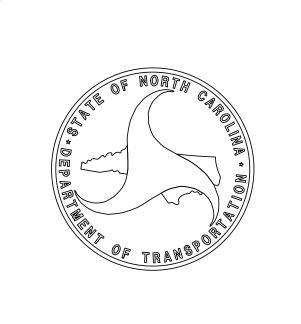
CORP. LICENSE NO.:

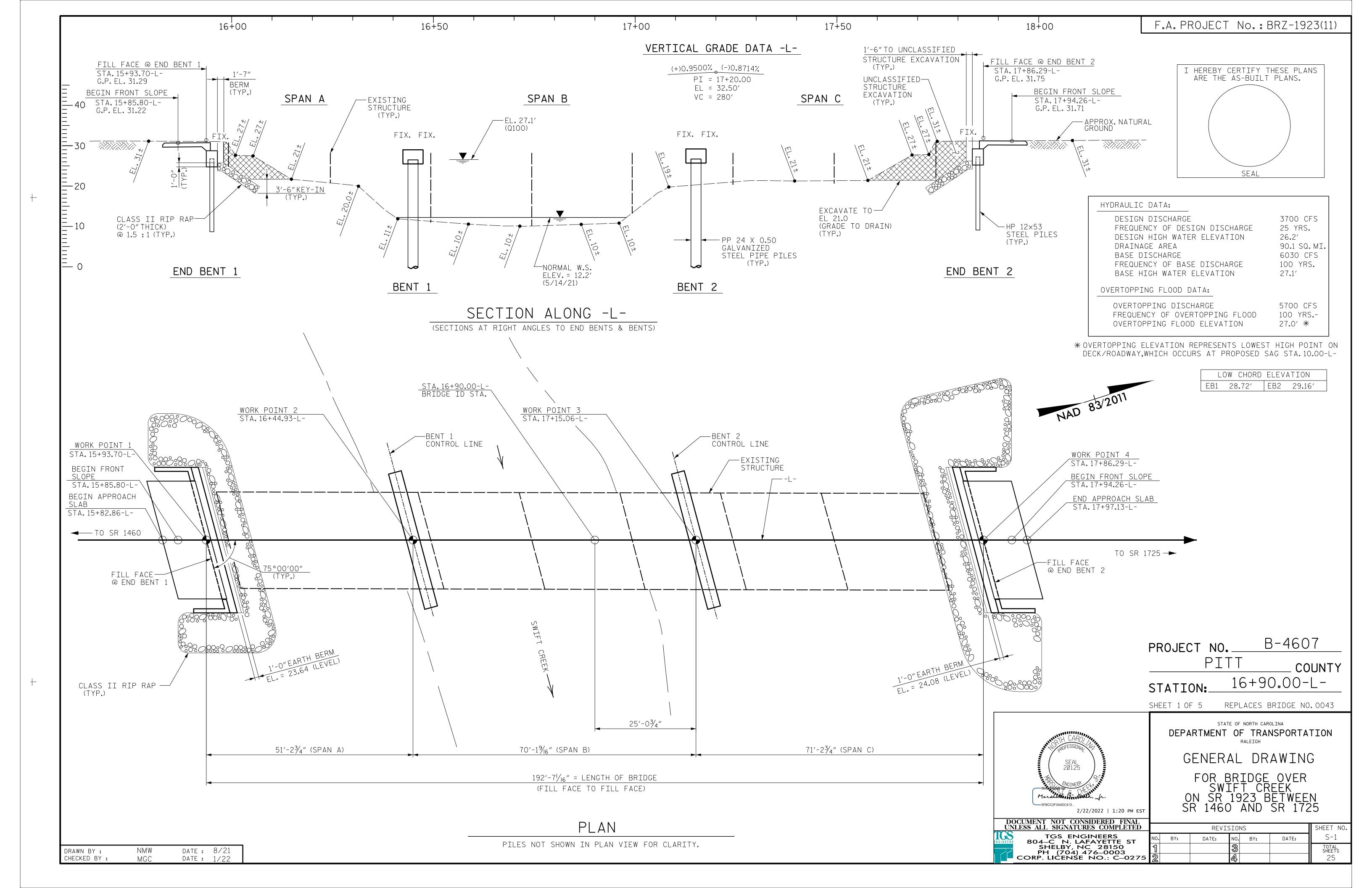
TO GARDNERVILLE

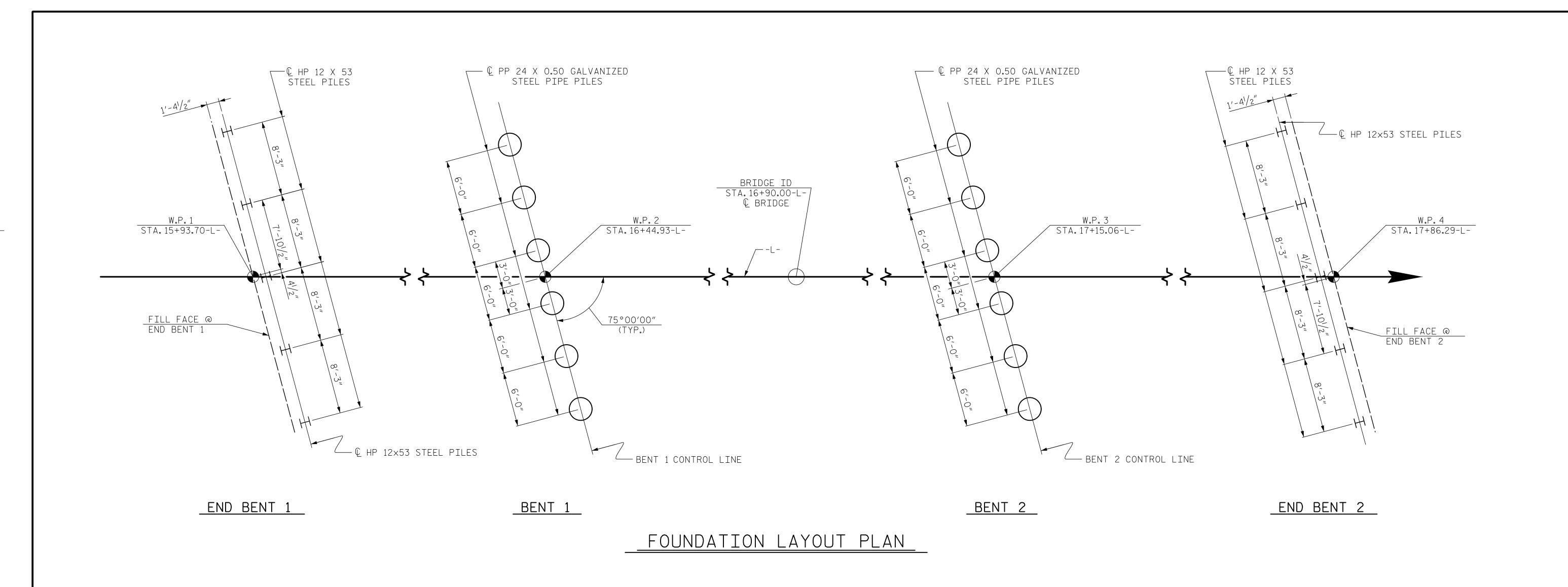


SEAL 20125 ENGINEER TOPE US GIVE ALL SFBCC2F3A4DC413...

SIGNATURE: 2/22/2022 | 1:20 PM EST







NOTES:

FOR PILES, SEE PILES PROVISION AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 65,000 TO 100,000 FT-LBS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT 1 AND BENT 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

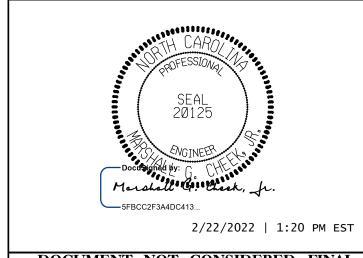
DO NOT INSTALL PILES AT END BENT NO.2 PRIOR TO THE UNDERCUT EXCAVATION AND BACKFILL TO THE BOTTOM OF CAP ELEVATION AT END BENT NO.2. SEE ROADWAY PLANS FOR UNDERCUT EXCAVATION.

PROJECT NO. B-4607

PITT COUNTY

STATION: 16+90.00-L-

SHEET 2 OF 5



DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER

SWIFT CREEK

ON SR 1923 BETWEEN

SR 1460 AND SR 1725

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
804-C N. LAFAYETTE ST
SHELBY, NC 28150
PH (704) 476-0003
CORP. LICENSE NO.: C-0275

REVISIONS SHEET NO.

BY: DATE: NO. BY: DATE: S-2

3 TOTAL SHEETS
25

DRAWN BY: ZCS DATE: 8/21 CHECKED BY: MGC DATE: 8/21

SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

| Find Boot/ | | | | | | Driven Piles | | | Predrilling for Piles* | | Γ | Orilled-In Piles | |
|--|--|--|---|--------------------------------------|---|---|---|---|---|---|--|--|---|
| End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5") | Factored Resistance per Pile TONS | Pile Cut-Off (Top of Pile) Elevation FT | Estimated Pile Lenth per Pile FT | Scour Critical Elevation FT | Min Pile Tip (Tip No Higher Than) Elev FT | Required Driving Resistance (RDR)** per Pile TONS | Total Pile Redrives Quantity EACH | Predrilling Length per Pile Lin FT | Predrilling Elevation (Elev Not To Predrill Below) FT | Maximum Predrilling Dia INCHES | Pile Excavation (Bottom of Hole) Elev FT | Pile Exc Not In Soil per Pile Lin FT | Pile Exc In Soil per Pile Lin FT |
| End Bent 1, Piles 1-5 | 85 | 26.60 | 85 | | | 195 | | | | | | | |
| Bent 1, Piles 1-6 | 165 | 27.50 | 105 | -2 | -27.0 | 230 | 11 total | | | | | | |
| Bent 2, Piles 1-6 | 185 | 27.70 | 110 | -1 | -27.0 | 290 | - 11 total | | | | | | |
| End Bent 2, Piles 1-5 | 100 | 27.10 | 85 | | | 170 | | | | | | | |

*Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

 $^{**}RDR = \frac{Factored\ Resistance +\ Factored\ Downdrag\ Load +\ Factored\ Dead\ Load}{Dynamic\ Resistance\ Factor} + Nominal\ Downdrag\ Resistance + \frac{Nominal\ Scour\ Resistance}{Scour\ Resistance\ Factor}$

PILE DESIGN INFORMATION

(Blank entries indicate item is not applicable to structure)

| End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5") | Factored Axial Load per Pile TONS | Factored Downdrag Load per Pile TONS | Factored Dead Load* per Pile TONS | Dynamic Resistance Factor | Nominal Downdrag Resistance per Pile TONS | Nominal Scour Resistance per Pile TONS | Scour Resistance Factor (Default = 1.00) |
|--|---|--|---|---------------------------------|---|---|---|
| End Bent 1, Piles 1-5 | 81 | 21.3 | | 0.60 | 16.6 | | |
| Bent 1, Piles 1-6 | 164 | | | 0.75 | | 5.5 | 1.00 |
| Bent 2, Piles 1-6 | 183 | | | 0.75 | | 41.0 | 1.00 |
| End Bent 2, Piles 1-5 | 87 | 0.8 | | 0.60 | 0.6 | | |

*Factored Dead Load is factored weight of pile above the ground line.

NOTES:

- 1. The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Jinyoung Park, PE# 032171) on 1-13-2022.
- 2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.
- 3. The Engineer will determine the need for PDA Testing when PDAs may be required.

PILE FOUNDATION TABLES

SUMMARY OF PDA/PILE ORDER LENGTHS

(Blank entries indicate item is not applicable to structure)

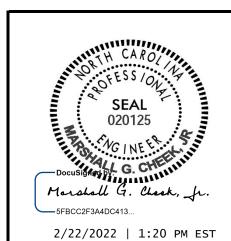
| Pi | le Driving Analyz | Pile Order Lengths | | | |
|-----------------------|-------------------|--------------------|---|-------------------------|--|
| End Bent/ Bent No | I Required? I | | Total PDA Testing Quantity EACH | End Bent/ Bent No(s) | Pile Order Length Basis* EST or PDA |
| End Bent 1, Piles 1-5 | MAYBE | 90 | | | |
| Bent 1, Piles 1-6 | YES | 110 | 2 | | |
| Bent 2, Piles 1-6 YES | | 115 | 3 | | |
| End Bent 2, Piles 1-5 | MAYBE | 90 | | | |

*EST = Pile order lengths from estimated pile lengths; PDA = Pile order lengths based on PDA testing. For groups of end bents/bents with pile order lengths based on PDA testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the PDA.

 PROJECT NO.
 B-4607

 PITT
 COUNTY

 STATION:
 16+90.00 -L



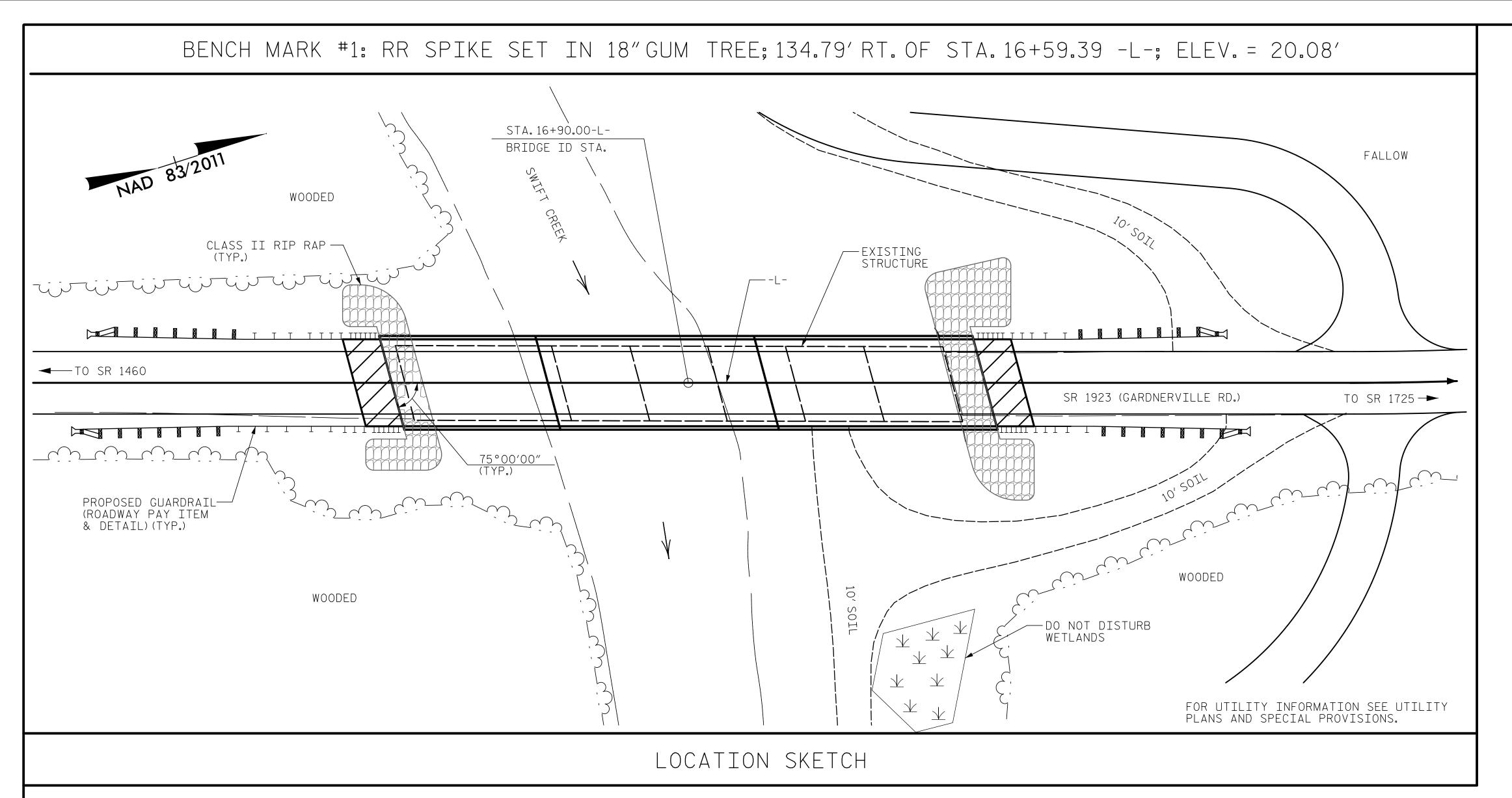
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER SWIFT CREEK ON SR 1923 BETWEEN SR 1460 AND SR 1725

| | | | SHEET NO. | | | | |
|------------------------|-----|-----|-----------|-----|-----|-------|--------|
| | | | S-3 | | | | |
| OCUMENT NOT CONSIDERED | NO. | BY: | DATE: | NO. | BY: | DATE: | TOTAL |
| FINAL UNLESS ALL | 1 | | | 3 | | | SHEETS |
| SIGNATURES COMPLETED | 2 | | | 4 | | | 25 |

SHEET 3 OF 5



NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE EXISTING 7 SPAN STRUCTURE (1 @ 25'-4'', 5 @ 25'-0'', 1 @ 25'-4'') CONSISTING OF A REINFORCED CONCRETE DECK ON STEEL I-BEAMS WITH A CLEAR ROADWAY WIDTH OF 24'-0'' AND 4/2'' ASPHALT WEARING SURFACE; WITH A SUBSTRUCTURE CONSISTING OF REINFORCED CONCRETE CAPS AND TIMBER PILES AT THE END BENTS AND BENTS WITH STEEL CRUTCH BENTS AT BENTS 2, 3, AND 4 AND LOCATED AT THE SITE OF THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THIS LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR THE DISTANCE OF 25 FT.LT. & 25 FT.RT. @ EB1 AND 30 FT.LT. AND 30 FT.RT. @ EB2 OF THE CENTERLINE OF THE BRIDGE AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION, SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITIES ON ROADWAY PLANS.

- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES".
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD. THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 16+90.00-L-."

AT THE CONTRACTOR'S OPTION, PRESTRESSED CONCRETE END BENT AND BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO RECEIVE PLANS AND DETAILS FROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY ADDITIONAL MATERIALS NEEDED WILL BE AT NO ADDITIONAL COST TO THE CONTRACTOR.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE SAMPLE BARS SHOULD COME FROM STEEL ACTUALLY USED IN THE PROJECT AND THE SAMPLE BARS SHOULD BE REPLACED BY SPICED BARS AS SPECIFIED IN THE SAMPLE BAR REPLACEMENT CHART. PAYMENT FOR THE SAMPLE BARS AND REPLACEMENT REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

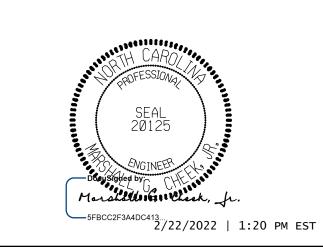
| | SAMPLI REPLAC | | | | |
|------|------------------|------|---------|--|--|
| SIZE | LENGTH | SIZE | LENGTH | | |
| #3 | 6'-2" | #8 | 12'-0" | | |
| #4 | 7′-4″ | #9 | 13′-2″ | | |
| #5 | 8′-6″ | #10 | 14'-6" | | |
| #6 | 9′-8″ | #11 | 15′-10″ | | |
| #7 | 10'-10" | | | | |

NOTE:
SAMPLE BAR REPLACEMENT
LENGTHS BASED ON
30"(SAMPLE LENGTH)
PLUS TWO SPLICE LENGTHS
AND f, = 60ksi.

PROJECT NO. B-4607
PITT COUNTY

STATION: 16+90.00-L-

SHEET 4 OF 5



DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER SWIFT CREEK ON SR 1923 BETWEEN SR 1460 AND SR 1725

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
804-C N. LAFAYETTE ST
SHELBY, NC 28150
PH (704) 476-0003
CORP. LICENSE NO.: C-0275

| | SHEET NO. | | | | |
|-----|-----------|-----------|-----|-------|-----------------|
| BY: | DATE: | NO. | BY: | DATE: | S-4 |
| | | જી | | | TOTAL SHEETS |
| | | 4 | | | 25 |

DRAWN BY: NMW DATE: 8/21 CHECKED BY: MGC DATE: 8/21

| | TOTAL BILL OF MATERIAL | | | | | | | | | |
|----------------|-------------------------------------|------------------------|----------------|---|-----------------------|-----------------------------|----------------------|---|---|--|
| ITEM | REMOVAL OF EXISTING STRUCTURE | ASBESTOS ASSESSMENT | PDA TESTING | UNCLASSIFIED STRUCTURE EXCAVATION | CLASS "A" CONCRETE | BRIDGE APPROACH SLABS | REINFORCING STEEL | PILE DRIVING EQUIPMENT SETUP FOR HP 12 x 53 STEEL PILES | PILE DRIVING EQUIPMENT SETUP FOR PP 24 × 0.50 GALVANIZED STEEL PIPE PILES | |
| | LUMP SUM | LUMP SUM | EA. | LUMP SUM | C.Y. | LUMP SUM | LBS. | EA. | EA. | |
| SUPERSTRUCTURE | | | | | | | | | | |
| END BENT 1 | | | | | 20.7 | | 2,522 | 5 | | |
| BENT 1 | | | | | 18.0 | | 2,367 | | 6 | |
| BENT 2 | | | | | 18.0 | | 2 , 367 | | 6 | |
| END BENT 2 | | | | | 20.7 | | 2,522 | 5 | | |
| TOTALS | LUMP SUM | LUMP SUM | 3 | LUMP SUM | 77.4 | LUMP SUM | 9,778 | 10 | 12 | |

| TOTAL BILL OF MATERIAL | | | | | | | | | | | |
|------------------------|----------------------------|---------|---|----------|------------------|--------------------------------------|-------------------------------------|-------------------------------|-------------------------|------------|--|
| ITEM | HP 12×53 STEEL PILES | | PP 24 X 0.50 GALVANIZED STEEL PIPE PILES | | PILE REDRIVES | VERTICAL CONCRETE BARRIER RAIL | RIP RAP, CLASS II (2'-0"THK.) | GEOTEXTILE FOR DRAINAGE | ELASTOMERIC BEARINGS | PRES CO | "× 2'-0" STRESSED NCRETE ED SLABS |
| | NO. | LIN.FT. | NO. | LIN. FT. | EA. | LIN.FT. | TONS | S.Y. | LUMP SUM | NO. | LIN.FT. |
| SUPERSTRUCTURE | | | | | | 380.52 | | | | 30 | 1900.00 |
| END BENT 1 | 5 | 425 | | | | | 160 | 180 | | | |
| BENT 1 | | | 6 | 630 | | | | | | | |
| BENT 2 | | | 6 | 660 | | | | | | | |
| END BENT 2 | 5 | 425 | | | | | 190 | 210 | | | |
| TOTALS | 10 | 850 | 12 | 1290 | 11 | 380.52 | 350 | 390 | LUMP SUM | 30 | 1900.00 |

DATE: 8/21 DATE: 2/22

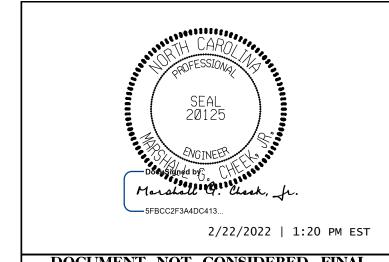
ZCS MGC

DRAWN BY : CHECKED BY : PROJECT NO. B-4607

PITT COUNTY

STATION: 16+90.00-L-

SHEET 5 OF 5



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING
FOR BRIDGE OVER
SWIFT CREEK
ON SR 1923 BETWEEN
SR 1460 AND SR 1725

| DOCUMENT NOT CONSIDERED FINAL | |
|--|--------|
| UNLESS ALL SIGNATURES COMPLETED | |
| TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C-0275 | NO. 1 |
| CORP. LICENSE NO.: C-0275 | 2 |

| | SHEET NO. | | | | | | |
|-----|-------------------------|-----------|--|--|-----------------|--|--|
| BY: | BY: DATE: NO. BY: DATE: | | | | | | |
| | | જી | | | TOTAL SHEETS | | |
| | | A | | | 25 | | |

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT DISTRIBUTION FACTORS (DF) DISTRIBL FACTORS DIS. FAC CII 0.28 1.84 24.48 0.61 0.80 0.28 1.66 50′ 2.18 50′ 50′ HL-93(Inv)N/A 1.66 1.75 EL EL 2.48 24.48 2.38 1.35 0.28 2.38 50′ 24.48 0.61 2.87 HL-93(0pr) N/A EL 50′ EL 2.48 N/A DESIGN LOAD 36.000 2.06 74.16 0.28 50′ EL 0.61 2.63 50′ EL 0.28 2.06 50′ HS-20(Inv) 2 1.75 2.28 24.48 2.48 0.80 24.48 EL RATING 106.56 24.48 36.000 0.28 50′ 50′ 2.96 2.96 EL 0.61 3.45 EL 2.48 N/A HS-20(0pr) 13.500 0.28 0.61 7.72 0.80 0.28 24.48 56.43 50′ 24.48 50′ 50′ 4.18 1.4 5.80 EL EL 2.48 4.18 SNSH EL SNGARBS2 20.000 3.31 66.20 0.28 4.58 50′ 24.48 0.61 5.59 2.48 0.80 0.28 50′ 24.48 50′ EL 3.31 EL 22.000 70.84 0.28 50′ 24.48 0.61 5.23 0.80 3.22 SNAGRIS2 3.22 4.44 EL 50′ EL 2.48 0.28 50′ EL 24.48 1.4 27.250 0.28 50′ 0.61 50′ EL 0.80 50′ SNCOTTS3 2.09 56.95 2.89 EL 24.48 3.79 2.48 0.28 2.09 24.48 EL 34.925 63.56 0.28 2.52 0.61 3.23 50′ 50′ 50′ SNAGGRS4 1.82 EL 24.48 EL 2.48 0.80 0.28 1.82 24.48 1.4 EL 35.550 62.92 0.28 2.45 50′ 24.48 0.61 3.33 0.80 0.28 24.48 1.77 1.4 50′ EL 50′ SNS5A EL 2.48 EL 39.950 66.32 0.28 2.29 50′ 24.48 0.61 3.07 2.48 0.80 0.28 1.66 24.48 1.66 EL 50′ EL 50′ SNS6A 1.4 EL 42.000 66.36 2.19 24.48 0.61 3.05 0.80 SNS7B 0.28 50′ EL 50′ EL 2.48 0.28 1.58 50′ EL 24.48 LEGAL LOAD 66.99 50′ 50′ 50′ TNAGRIT3 33.000 2.03 0.28 2.81 EL 24.48 0.61 3.60 EL 2.48 0.80 0.28 2.03 24.48 1.4 EL RATING 33.075 2.05 67.80 0.28 2.84 24.48 0.61 3.46 0.80 0.28 2.05 24.48 50′ 50′ 50′ TNT4A EL EL 2.48 EL 41.600 1.70 70.72 0.28 2.36 50′ 24.48 0.61 3.32 50′ 2.48 0.80 0.28 1.70 24.48 EL EL 50′ TNT6A 1.4 EL 42.000 72.66 0.28 2.40 24.48 0.61 3.09 0.80 TNT7A 50′ EL 50′ EL 50′ I.S. 1.73 2.48 0.28 1.73 EL 24.48 2.92 42.000 1.80 75.60 0.28 2.50 50′ EL 24.48 0.61 50′ EL 2.48 0.80 0.28 1.80 50′ 24.48 TNT7B 1.4 EL TNAGRIT4 43.000 73.53 0.28 2.37 50′ 24.48 0.61 2.81 50′ 0.80 50′ 1.71 EL EL 2.48 0.28 1.71 24.48 EL 45.000 1.60 72.00 0.28 2.22 50′ 24.48 0.61 2.85 2.48 0.80 0.28 1.60 24.48 EL 50′ EL 50′ TNAGT5A 1.4 EL

LOAD FACTORS:

DESIGN LOAD STRENGTH I 1.25 1.50 SERVICE III 1.00 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2

フ

1

EL **24.48**

EL 2.48 0.80 0.28 **1.57**

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

 $\langle 2 \rangle$ DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

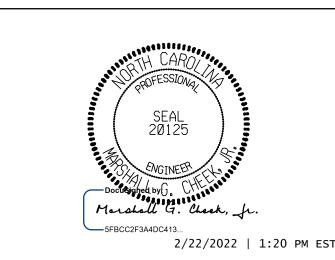
I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-4607
PITT COUNTY

STATION: 16+90.00-L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

LRFR SUMMARY FOR 50' CORED SLAB UNIT 75° SKEW

(NON-INTERSTATE TRAFFIC)

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

REVISIONS

SHEET NO.
BY: DATE: NO. BY: DATE:

3 TOTAL SHEETS
25

3

1.4 0.28 2.17

50′

EL 24.48 0.61 2.66 50'

LRFR SUMMARY

FOR SPAN 'A'

ASSEMBLED BY: ZCS DATE: 8/21 CHECKED BY: STM DATE: 8/21 DESIGN ENGINEER OF RECORD: ZCS DATE: 2/22

1.57 70.65

45.000 **3**

TNAGT5B

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS SERVICE III LIMIT STATE STRENGTH I LIMIT STATE MOMENT SHEAR MOMENT CONTROLLING LOAD RATING DISTRIBU[.] Factors (LIVELOAD FACTORS MIN RAT (RF) \Box \Box 34.482 0.608 0.80 1.014 1.75 0.269 1.04 0.269 1.01 34.482 N/A 70′ EL 1.1 70′ EL 3.448 70′ HL-93(Inv)EL 1.35 1.355 70′ EL 34.482 0.608 70′ EL N/A HL-93(0pr) N/A 1.35 0.269 1.43 3.448 ___ DESIGN LOAD 47.356 36.000 1.315 0.608 1.38 0.80 0.269 1.32 HS-20(Inv)0.269 70′ EL 34.482 70′ EL 3.448 70′ EL 34.482 RATING 36.000 1.757 63.236 0.269 70′ 34.482 0.608 1.79 70′ 3.448 HS-20(0pr) 1.35 1.76 EL N/A 13.500 2.938 39.656 0.269 34.482 0.608 4.12 0.80 0.269 2.94 SNSH 3.78 70′ EL 70′ EL 3.448 70′ 34.482 EL 0.80 20.000 2.203 44.052 2.93 70′ 0.269 2.20 SNGARBS2 70′ EL 34.482 0.608 EL 3.448 70′ 0.269 2.84 34.482 EL 22.000 46.016 0.608 0.80 0.269 2.09 SNAGRIS2 70′ EL 34.482 70′ EL 3.448 70′ 34.482 2.092 0.269 2.69 2.72 EL 27.250 1.462 39.844 0.608 2.06 0.80 0.269 1.46 SNCOTTS3 0.269 1.88 70′ ΕL 34.482 70′ EL 3.448 70′ 34.482 EL 34.925 42.856 34.482 0.608 0.80 SNAGGRS4 1.227 0.269 1.58 70′ EL 1.71 70′ EL 3.448 0.269 1.23 70′ 34.482 35.550 42.646 0.269 1.54 34.482 0.608 1.73 70′ 0.80 0.269 1.20 70′ EL EL 3.448 70′ 34.482 SNS5A EL 39.950 44.058 0.608 1.58 0.80 0.269 1.10 70′ EL 34.482 70′ EL 3.448 70′ 34.482 SNS6A 0.269 EL 1.55 0.80 0.269 44.113 70′ 34.482 0.608 70′ 3.448 1.05 70′ SNS7B 42.000 1.05 0.269 1.35 EL EL EL 34.482 LEGAL LOAD 33.000 44.401 34.482 0.608 0.80 0.269 1.88 0.269 1.35 TNAGRIT3 1.345 1.73 70′ EL 70′ EL 3.448 70′ EL 34.482 RATING 0.80 TNT4A 33.075 1.352 44.717 0.269 1.74 70′ EL 34.482 0.608 1.83 70′ EL 3.448 0.269 1.35 70′ 34.482 EL 41.600 34.482 0.608 1.65 0.80 0.269 70′ EL 70′ EL 3.448 70′ 34.482 TNT6A 0.269 1.43 1.11 EL

70′

70′

70′

70′

70′

EL

EL

EL

EL

1.43

1.49

1.41

1.31

0.608

0.608

0.608

0.608

34.482

34.482

34.482

34.482

EL | 34.482 | 0.608 |

1.62

1.51

1.46

1.45

70′

70′

70′

70′

EL

EL

EL

EL

0.80

0.80

0.80

0.80

3.448 | 0.80 | 0.269 |

3.448

3.448

3.448

3.448

0.269

0.269

0.269

0.269

1.11

1.16

1.10

1.03

1.02

70′

70′

70′

70′

70′

EL

EL

EL

EL

34.482

34.482

34.482

34.482

EL **34.482**

LOAD FACTORS:

| | DESIGN LOAD RATING | LIMIT STATE | γ_{DC} | $\gamma_{\sf DW}$ |
|--|--------------------------|-------------|---------------|-------------------|
| | | STRENGTH I | 1.25 | 1.50 |
| | FACTORS | SERVICE III | 1.00 | 1.00 |

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2-

3

4.

(#) CONTROLLING LOAD RATING

 $\langle 1 \rangle$ DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

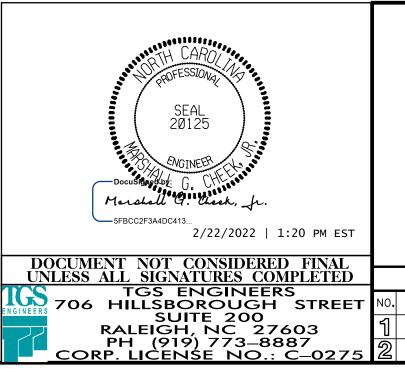
EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-4607

PITT COUNTY

STATION: 16+90.00-L-

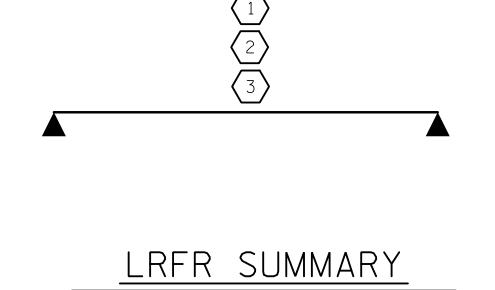


DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD LRFR SUMMARY FOR 70'CORED SLAB UNIT 75° SKEW

(NON-INTERSTATE TRAFFIC)

| | SHEET NO. | | |
|-----|-----------|--|-----------------|
| BY: | S-7 | | |
| | 3 | | TOTAL SHEETS |
| | 4 | | 25 |



FOR SPANS 'B' & 'C'

0.269

0.269

0.269

0.269

0.269 |

ASSEMBLED BY: ZCS DATE: 8/21 CHECKED BY: MGC DATE: 1/22

DRAWN BY: CVC 6/IO CHECKED BY: DNS 6/IO

46.794

48.526

47.174

46.505

45.905|

1.4

1.114

1.155

1.097

42.000

42.000

43.000

45.000

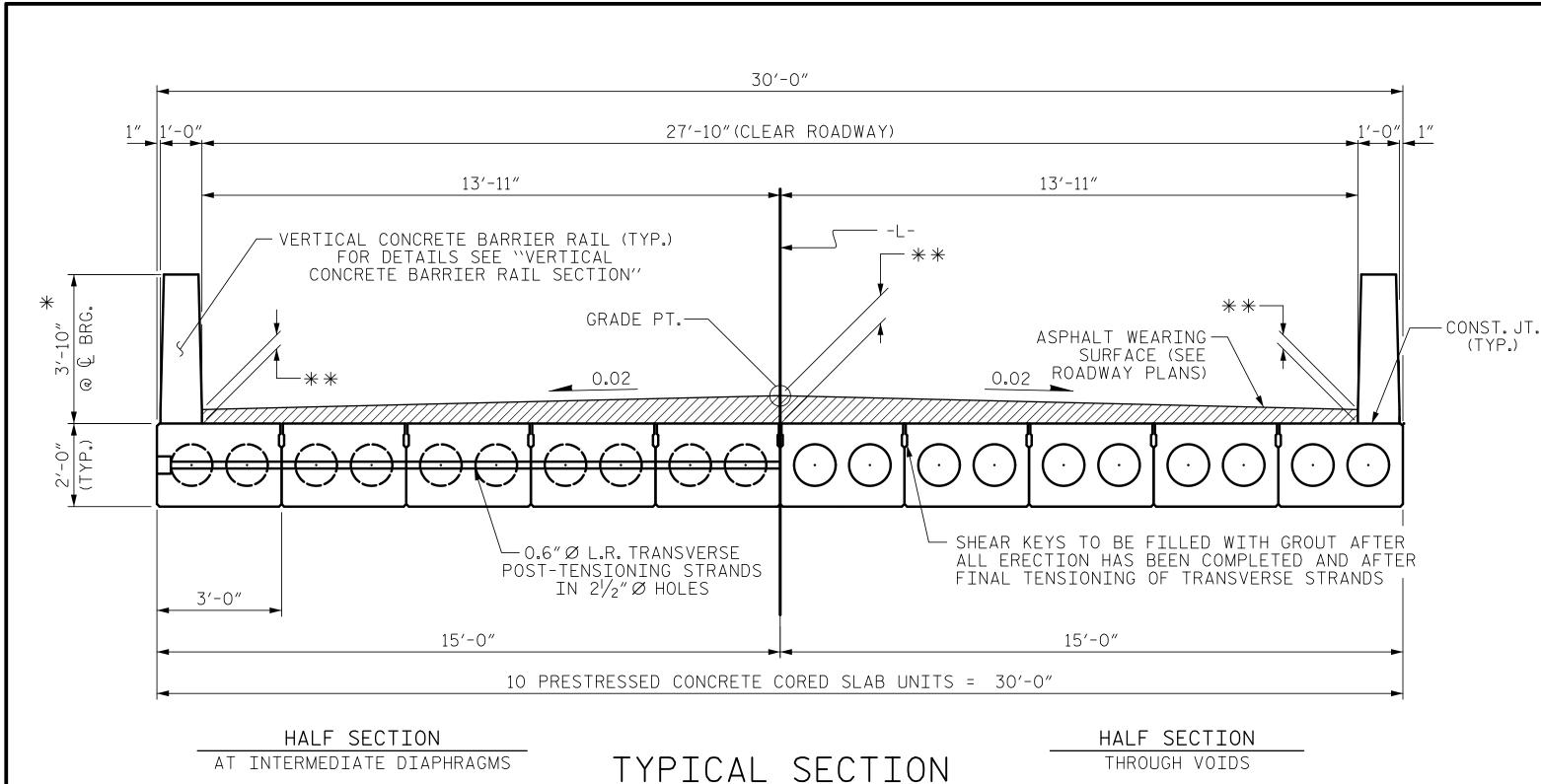
TNT7A

TNT7B

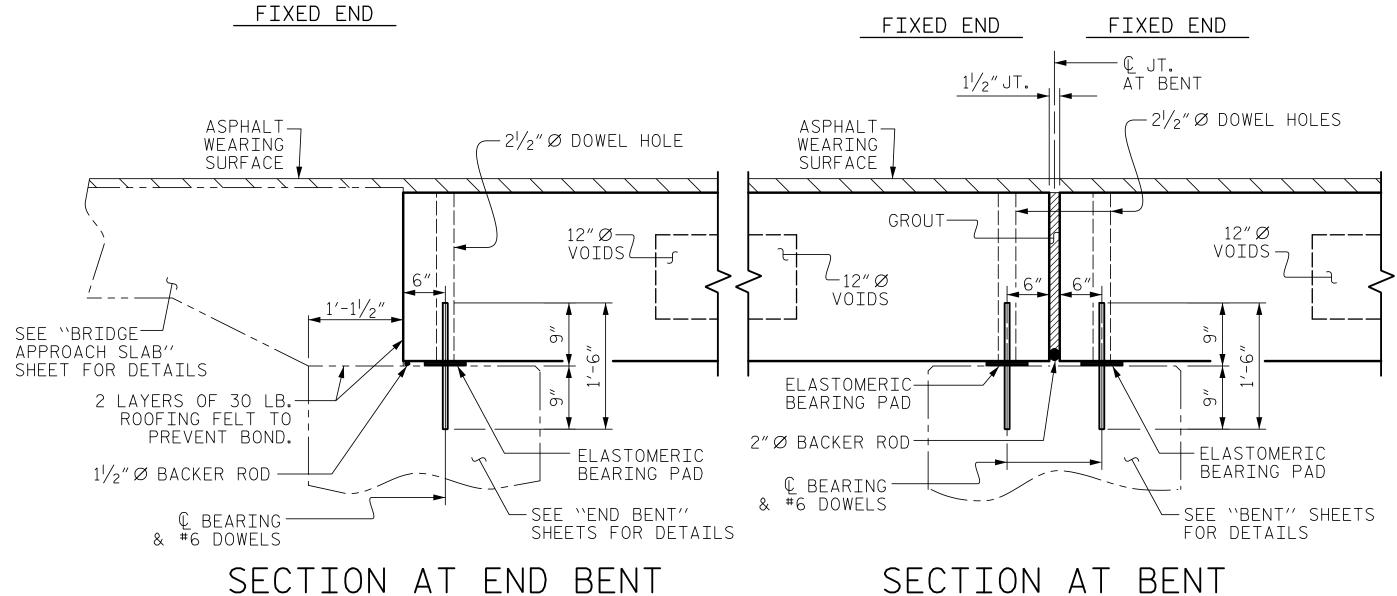
TNAGRIT4

TNAGT5A

TNAGT5B



*- THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.



SECTION AT END BENT

PERMITTED THREADED INSERT

THREADED INSERT DETAIL

CHECKED BY: MGC DATE: 1/22
DESIGN ENGINEER OF RECORD: ZCS DATE: 2/22

DATE: 9/21

CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND

RECESSED 3/8". SIZE TO BE

DETERMINED

ASSEMBLED BY: NMW/ZCS

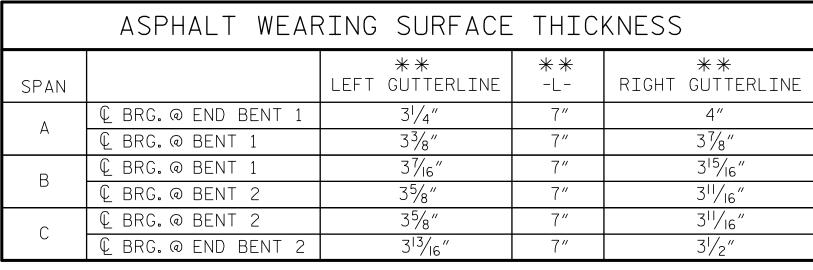
BY CONTRACTOR. —

€ 0.6″Ø L.R. TRANSVERSE POST-TENSIONING STRAND HOLE FOR SHEATHED WITH A <u></u> − 5/8" × 5" × 10" ₽ NON-CORROSIVE PIPE. — TRANSVERSE STRAND STRAND VISE 51/4" × 101/4" FILL RECESS
WITH GROUT OF EXTERIOR CORED SLAB ELEVATION VIEW SECTION B-B

GROUTED RECESS AT END OF POST-TENSIONED STRAND CORED SLABS

ASPHALT WEARING SURFACE THICKNESS *** * RIGHT GUTTERLINE SPAN LEFT GUTTERLINE -L-3¹/₄" BRG. @ END BENT 3 1/8" 33/8" 7" BRG. @ BENT 1 3½6″ 3¹⁵/₁₆" 7" 🗋 BRG. @ BENT 1 3¹¹/₁₆" 35/8" 7" È BRG. @ BENT 2 35/8" 311/16" BRG. @ BENT 2 7"

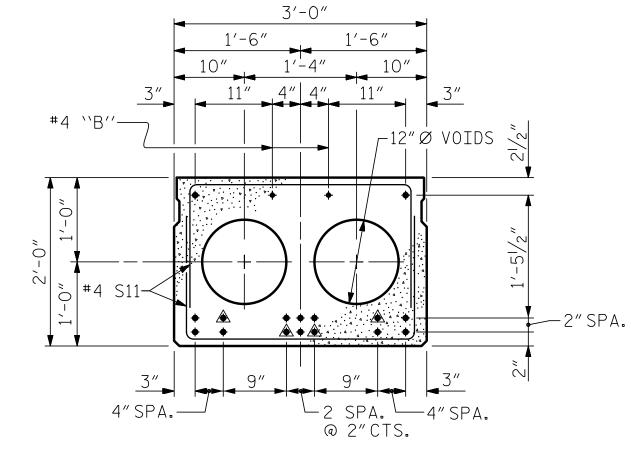
33/8" CL.



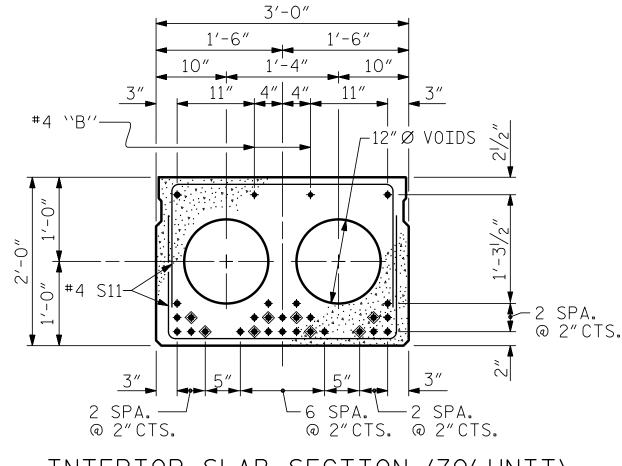
1'-4"

EXTERIOR SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)



INTERIOR SLAB SECTION (50' UNIT) (16 STRANDS REQUIRED)



INTERIOR SLAB SECTION (70' UNIT) (28 STRANDS REQUIRED)

RELAXATION STRAND LAYOUT

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

SHEET 1 OF 5

SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.

B-4607 PROJECT NO. _ COUNTY

STATION: 16+90.00-L-

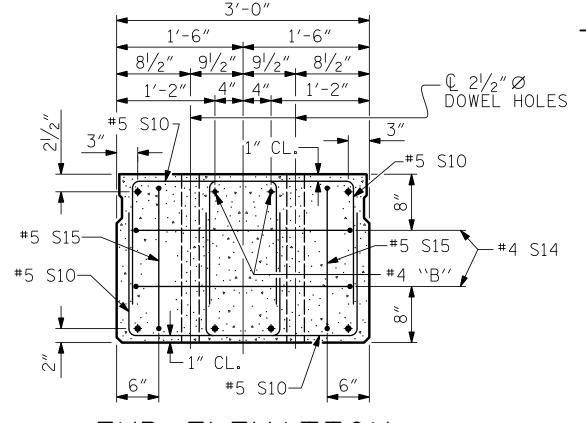
3'-0'' X 2'-0''

PRESTRESSÉD CONCRETE CORED SLAB UNIT

STATE OF NORTH CAROLINA

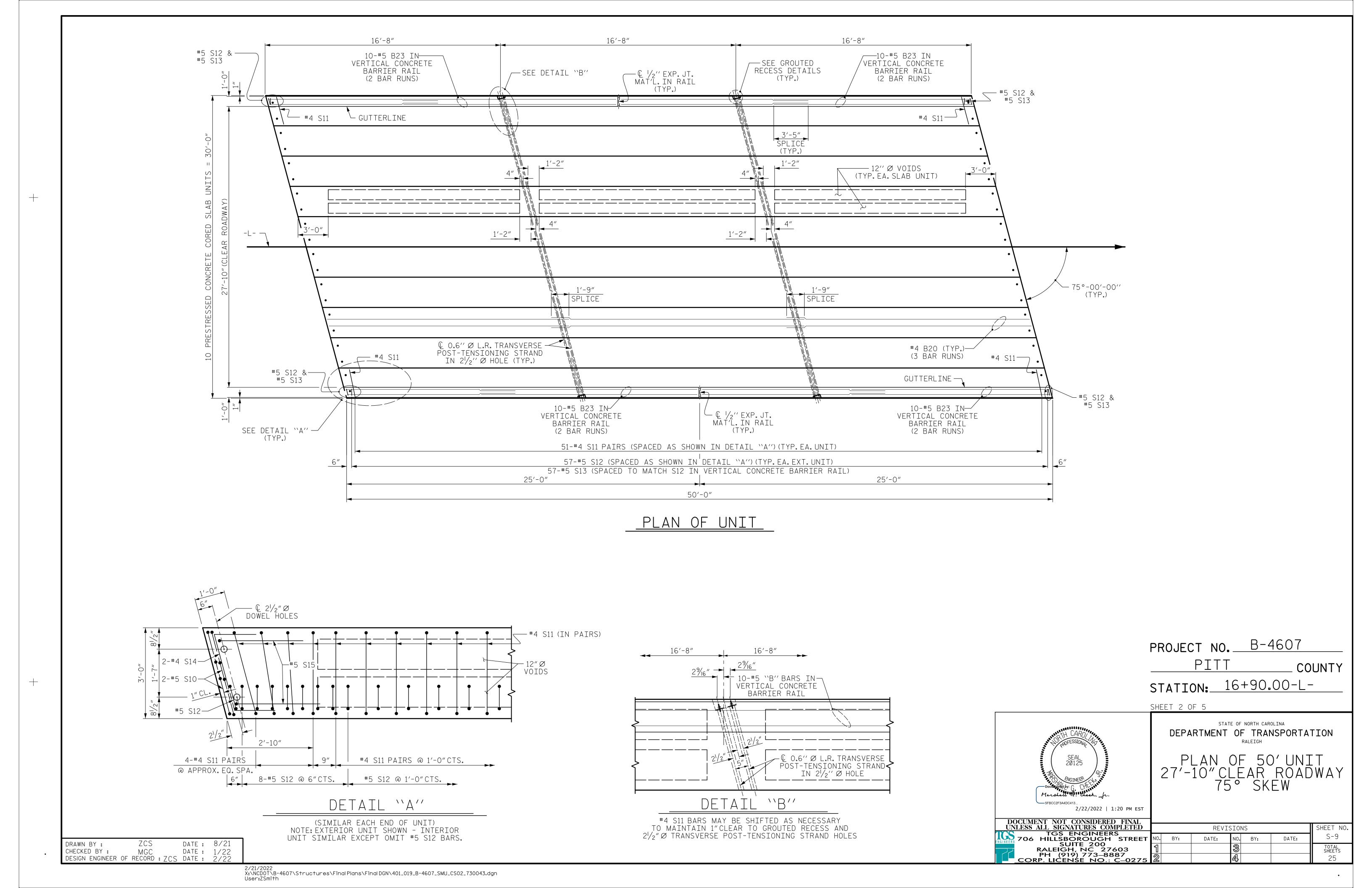
DEPARTMENT OF TRANSPORTATION

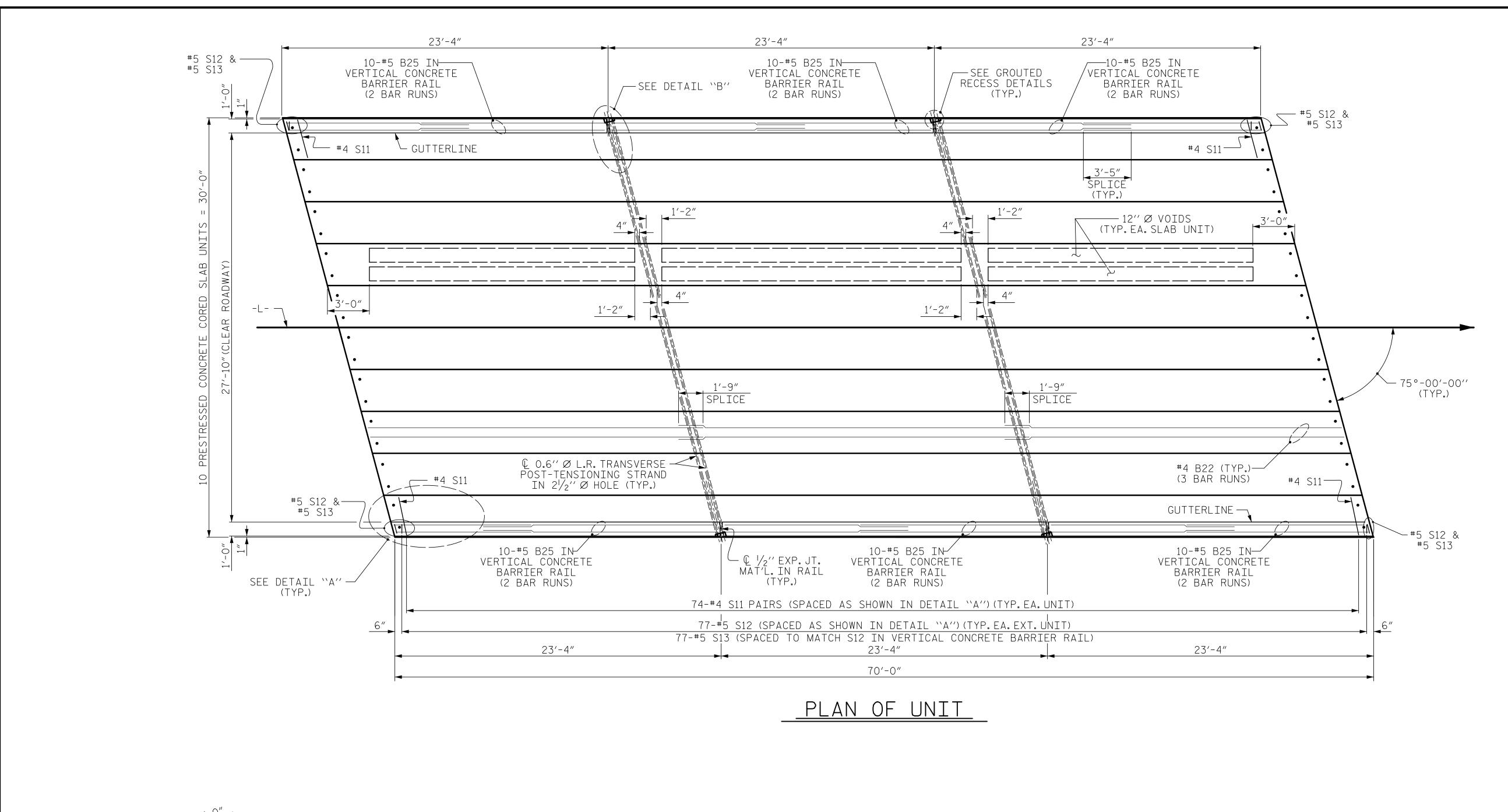
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED SHEET NO **REVISIONS** TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275 S-8 DATE: DATE: BY: BY: TOTAL SHEETS

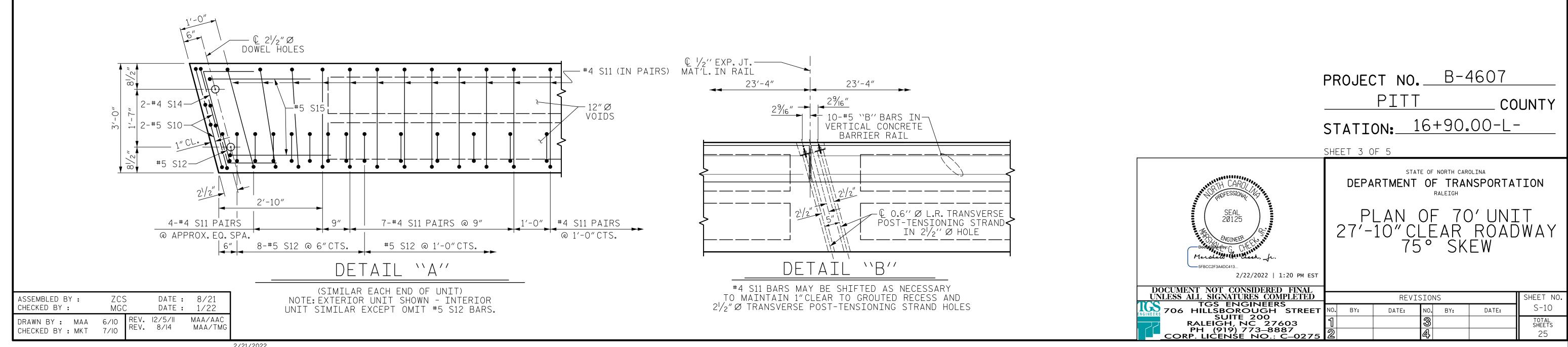


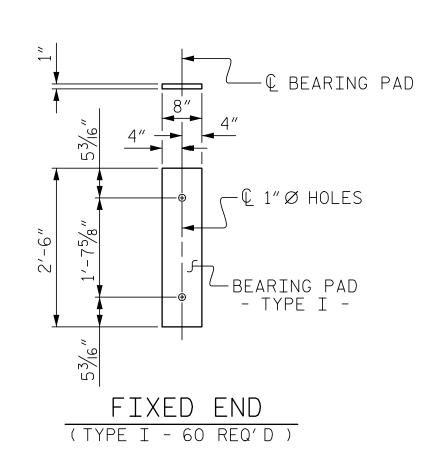
END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.









ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

| CONCRETE RELEA | ASE STRENGTH |
|----------------|--------------|
| | |
| UNIT | PSI |
| 50'UNITS | 5000 |
| 70'UNITS | 5500 |

| GRADE 270 S | TRANDS | |
|---------------------------------------|------------|--|
| | 0.6″Ø L.R. | |
| AREA (SQUARE INCHES) | 0.217 | |
| ULTIMATE STRENGTH (LBS.PER STRAND) | 58,600 | |
| APPLIED PRESTRESS (LBS.PER STRAND) | 43,950 | |

50'UNIT

80

118

CLASS AA CONCRETE

* EPOXY COATED REINFORCING STEEL

OTAL VERTICAL CONCRETE BARRIER RAIL

₩B23

₩B25

₩ S13

| BILL OF MATERIAL FOR ONE 50'CORED SLAB UNIT | | | | | | | |
|--|-----------|--------|---------|---------|---------|---------|---------|
| | | | | EXTERI | OR UNIT | INTERIO | OR UNIT |
| BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT | LENGTH | WEIGHT |
| B20 | 6 | #4 | STR | 17′-10″ | 71 | 17′-10″ | 71 |
| | | | | | | | |
| S10 | 8 | #5 | 3 | 4'-10" | 40 | 4'-10" | 40 |
| S11 | 102 | #4 | 3 | 5′-10″ | 397 | 5′-10″ | 397 |
| * S12 | 59 | #5 | 1 | 6′-0″ | 369 | | |
| S14 | 4 | #4 | 4 | 5′-8″ | 15 | 5′-8″ | 15 |
| S15 | 4 | #5 | 3 | 7′-1″ | 30 | 7′-1″ | 30 |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |
| REINFO | ORCING : | STEEL | LBS |) . | 553 | | 553 |
| ₩ EPOX | Y COATE | ED | | | | | |
| REIN | IFORCINO | STEEL | LBS | 5. | 369 | | |
| 7000 F | P.S.I. CO | NCRETE | CU. YDS |) , | 8.8 | | 8.8 |
| | | | | | | | |
| 0.6″Ø | L.R. STR | ANDS | No |) , | 16 | | 16 |

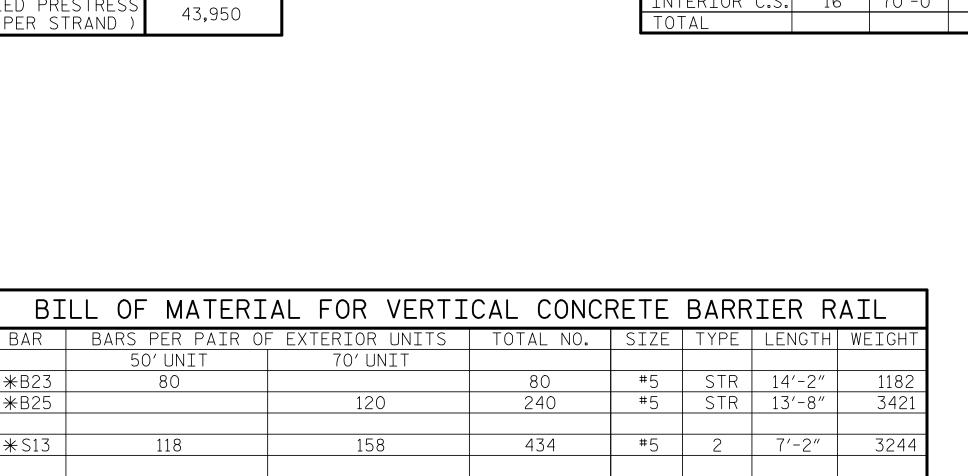
| BILL OF MATERIAL FOR ONE 70'CORED SLAB UNIT | | | | | | | |
|--|------------------------------------|------|------|--------|---------|---------|---------|
| | | | | EXTERI | OR UNIT | INTERI(| OR UNIT |
| BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT | LENGTH | WEIGHT |
| B22 | 6 | #4 | STR | 24'-6" | 98 | 24'-6" | 98 |
| | | | | | | | |
| S10 | 8 | #5 | 3 | 4'-10" | 40 | 4'-10" | 40 |
| S11 | 148 | #4 | 3 | 5′-10″ | 577 | 5′-10″ | 577 |
| * S12 | 79 | #5 | 1 | 6′-0″ | 494 | | |
| S14 | 4 | #4 | 4 | 5′-8″ | 15 | 5′-8″ | 15 |
| S15 | 4 | #5 | 3 | 7'-1" | 30 | 7'-1" | 30 |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |
| REINF(| REINFORCING STEEL LBS. 760 760 | | | | | 760 | |
| | * EPOXY COATED | | | | | | |
| REINFORCING STEEL LBS. 494 | | | | | | | |
| 7000 1 | 7000 P.S.I. CONCRETE CU. YDS. 12.0 | | | | | | 12.0 |
| | | | | | | | |
| 0.6"Ø | L.R. STR | ANDS | No |) . | 28 | | 28 |

| CORED | CORED SLABS REQUIRED | | | | | |
|---------------|----------------------|--------|--------------|--|--|--|
| | NUMBER | LENGTH | TOTAL LENGTH | | | |
| 50'UNIT | | | | | | |
| EXTERIOR C.S. | 2 | 50'-0" | 100'-0" | | | |
| INTERIOR C.S. | 8 | 50'-0" | 400'-0" | | | |
| 70'UNIT | | | | | | |
| EXTERIOR C.S. | 4 | 70'-0" | 280'-0" | | | |
| INTERIOR C.S. | 16 | 70'-0" | 1120′-0″ | | | |
| TOTAL | | | 1900'-0" | | | |

7847

45.1

380.52



LBS.

CU.YDS.

LN. FT.

| BAR TYPE | ES |
|--|---|
| 7" 1,-10," | 3'-4" 7 ³ / ₄ " |
| S15 1'-8 \(\frac{1}{2}'' \) S11 2'-8" S10 1'-10" (3) \(\frac{1}{2} \) \(\frac{1} | 8 /4" 1'-6" 4 1'-6" 4 1'-6" 8 1'-6" |

| DEAD LOAD DEFLECTION AN | ND CAMBER |
|--|-----------------------------|
| | $3'-0'' \times 2'-0''$ |
| 50'CORED SLAB UNIT | 0.6″Ø L.R. STRAND |
| CAMBER (SLAB ALONE IN PLACE) | ¹ 1/16″ ♦ |
| DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD*** | 1/4″ ♦ |
| FINAL CAMBER | 7/16″ ♦ |
| ** INCLUDES FUTURE WEARING SURF | ACE |

| DEAD LOAD DEFLECTION AN | ND CAMBER |
|--|---------------------------------|
| | $3'-0'' \times 2'-0''$ |
| 70'CORED SLAB UNIT | 0.6″Ø L.R. STRAND |
| CAMBER (SLAB ALONE IN PLACE) | 2 ¹ / ₄ " |
| DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD*** | 3/4″ ♦ |
| FINAL CAMBER | 1/2" |
| ** INCLUDES FUTURE WEARING SURF | ACE |

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ " \alpha DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

| GUTTERLINE | ASPHALT T | THICKNESS | & RAIL | HEIGHT |
|------------|-----------------------------------|-------------------------|--------------------------------------|------------------|
| | | LAY THICKNESS)-SPAN | | HEIGHT)-SPAN |
| | LEFT | RIGHT | LEFT | RIGHT |
| SPAN A | 31/8" | 33/4" | 3'-91/8" | 3'-93/4" |
| SPAN B | 21/2" | 23/4" | 3'-81/2" | 3′-8¾″ |
| SPAN C | 2 ¹¹ / ₁₆ " | 29/16" | 3'-8 ¹¹ / ₁₆ " | 3′-89/16″ |
| | | | | |
| | | | 1 | |

B-4607 PROJECT NO. _ COUNTY STATION: 16+90.00-L-

5FBCC2F3A4DC413... 2/22/2022 | 1:20 PM EST

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

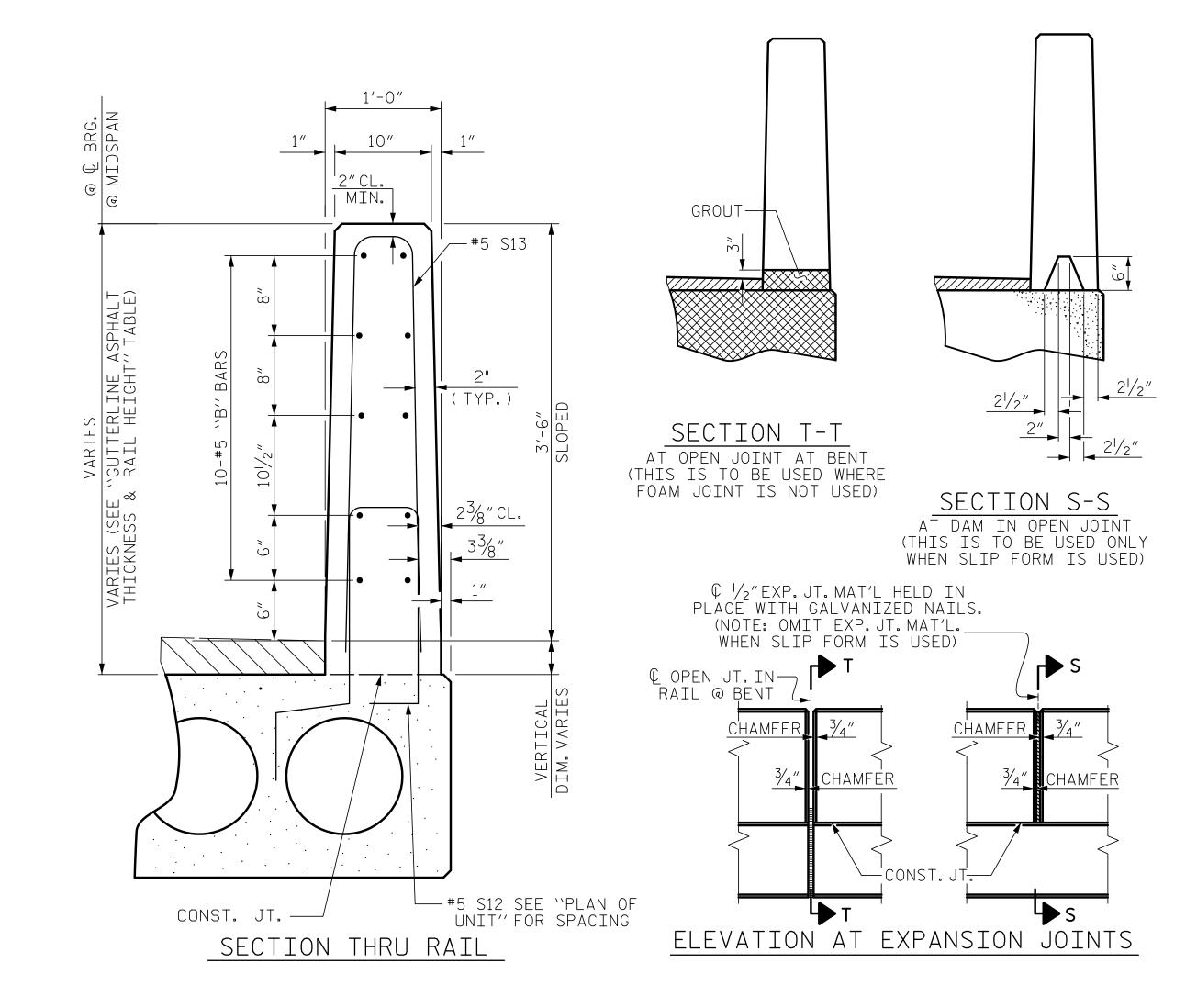
PRESTRESSED CONCRETE CORED SLAB UNIT

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200 RALEIGH, NC 27603 PH (919) 773–8887 CORP. LICENSE NO.: C-0275

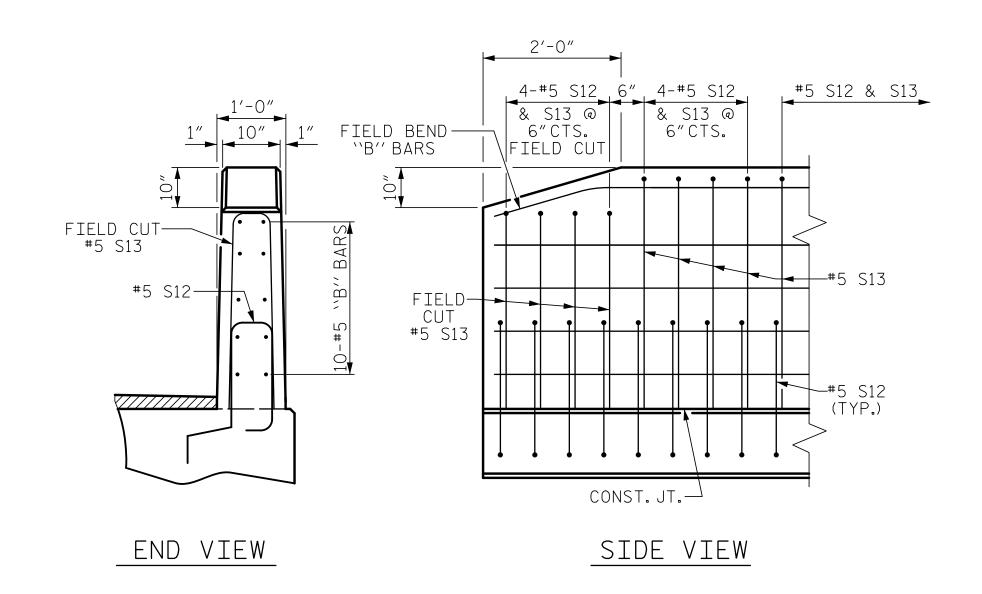
SHEET NO **REVISIONS** S-11 BY: DATE: DATE: BY: TOTAL SHEETS

SHEET 4 OF 5

ZCS DATE: 1/22 ASSEMBLED BY : CHECKED BY: DATE: 1/22 DRAWN BY: MAA 6/10 MAA/THC REV. 5/18 CHECKED BY: MKT 7/10



VERTICAL CONCRETE BARRIER RAIL DETAILS



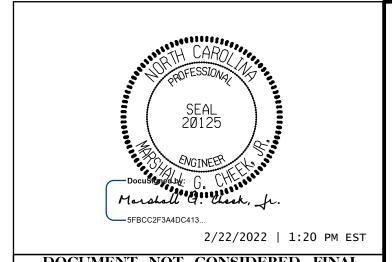
END OF RAIL DETAILS

PROJECT NO. B-4607

PITT COUNTY

STATION: 16+90.00-L-

SHEET 5 OF 5



DEPARTMENT OF TRANSPORTATION
RALEIGH

3'-0" X 2'-0" PRESTRESSED CONCRETE CORED SLAB UNIT

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

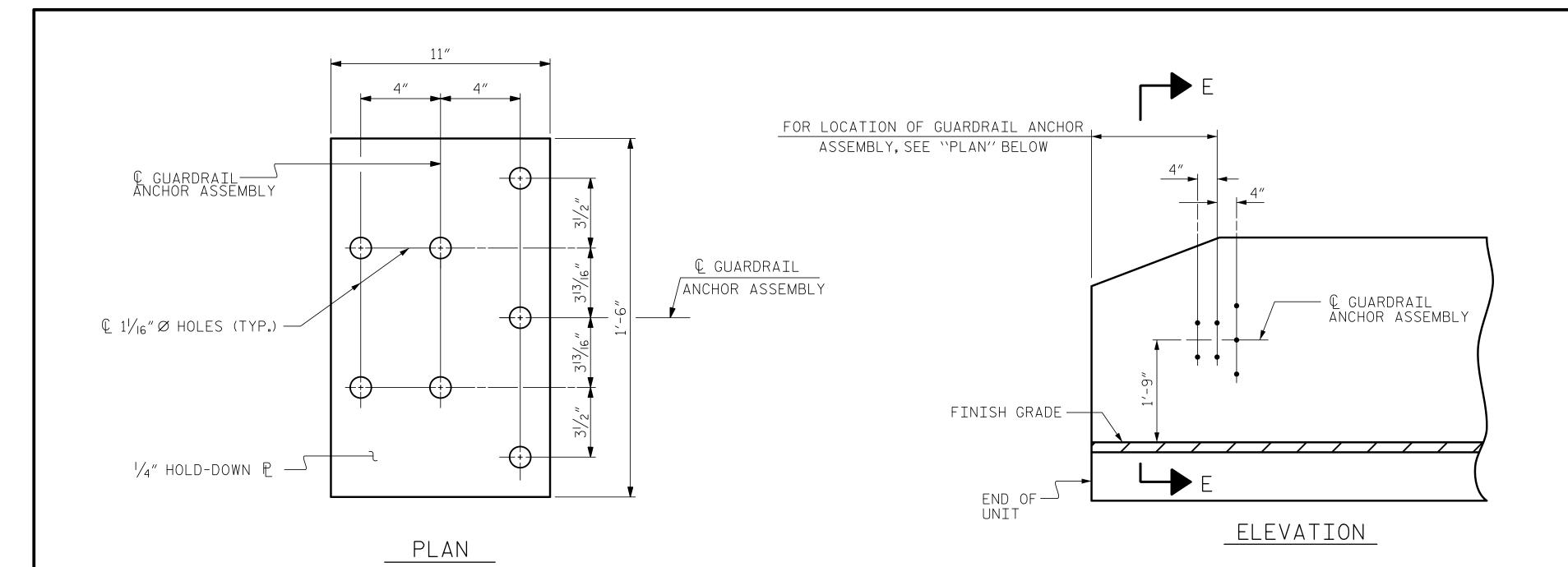
TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

REVISIONS

SHEET NO. BY: DATE: NO. BY: DATE: S-12

TOTAL SHEETS
25

DRAWN BY: ZCS DATE: 8/21
CHECKED BY: MGC DATE: 1/22
DESIGN ENGINEER OF RECORD: ZCS DATE: 2/22



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " \alpha GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

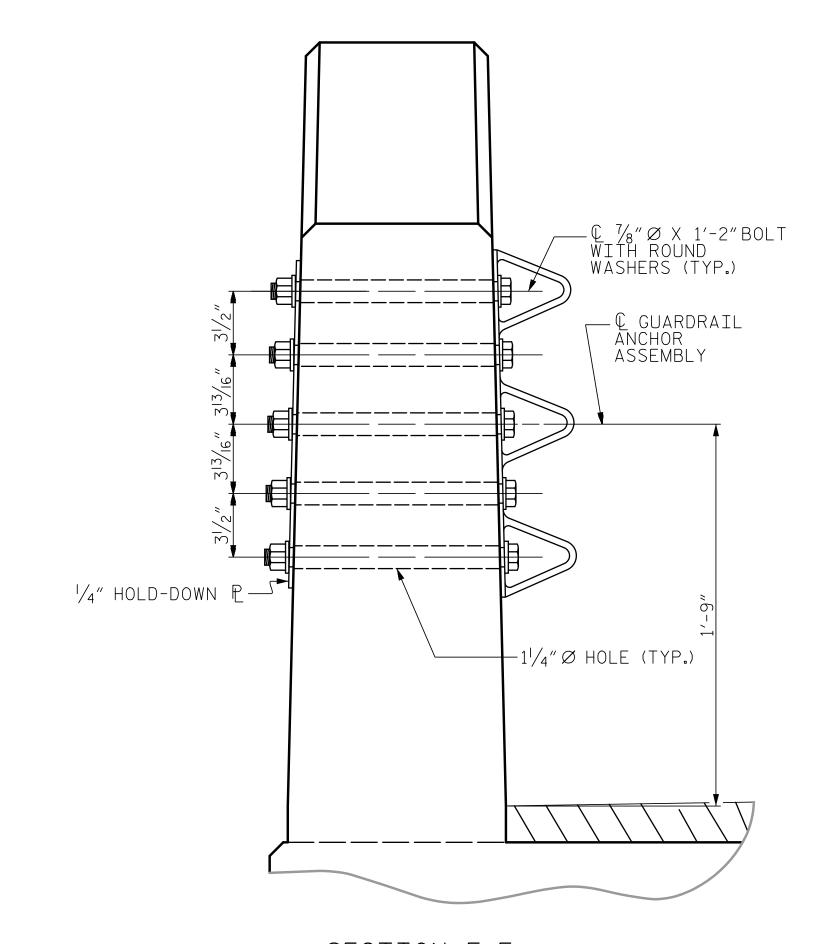
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

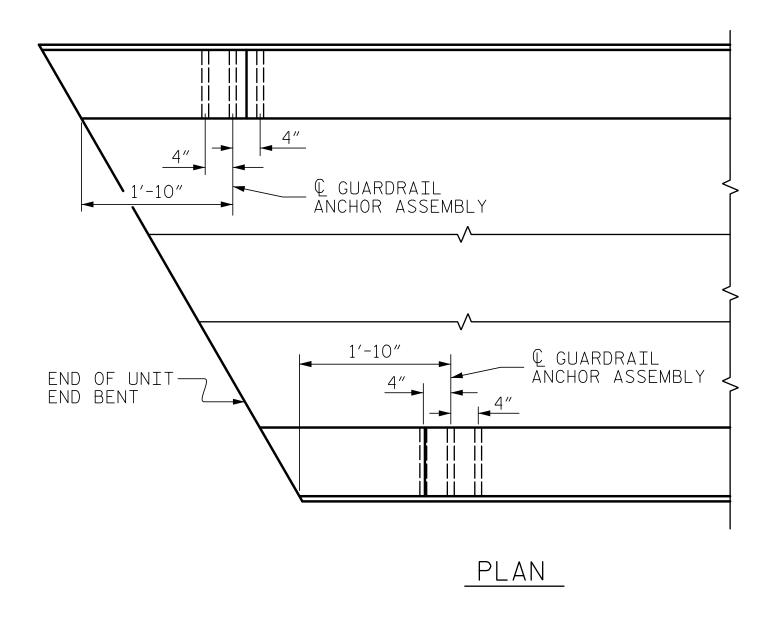
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

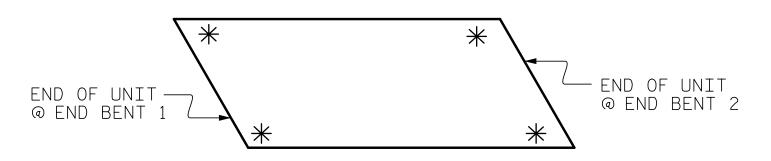


SECTION E-E GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

B-4607 PROJECT NO._ COUNTY STATION: 16+90.00-L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE

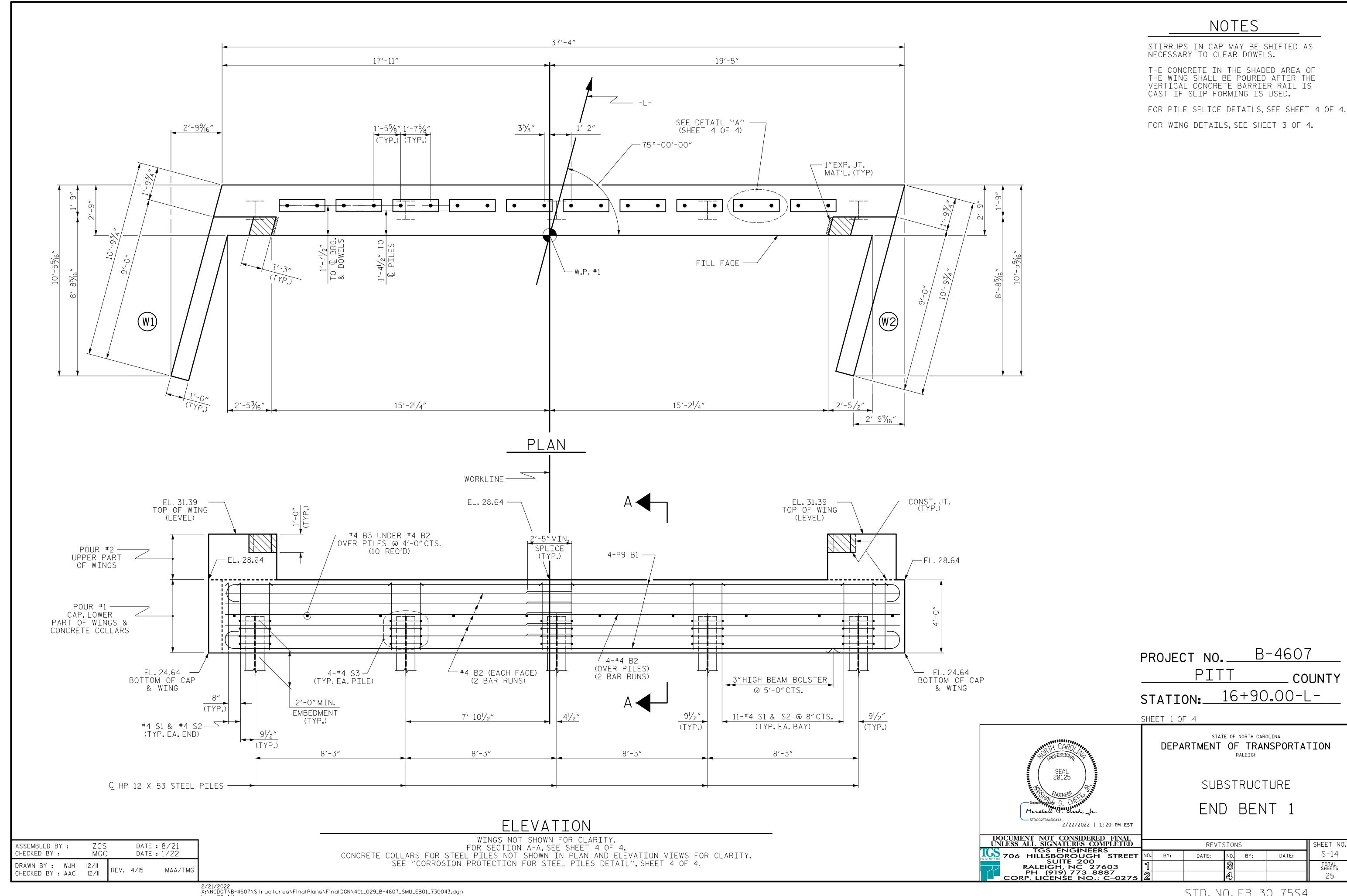
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

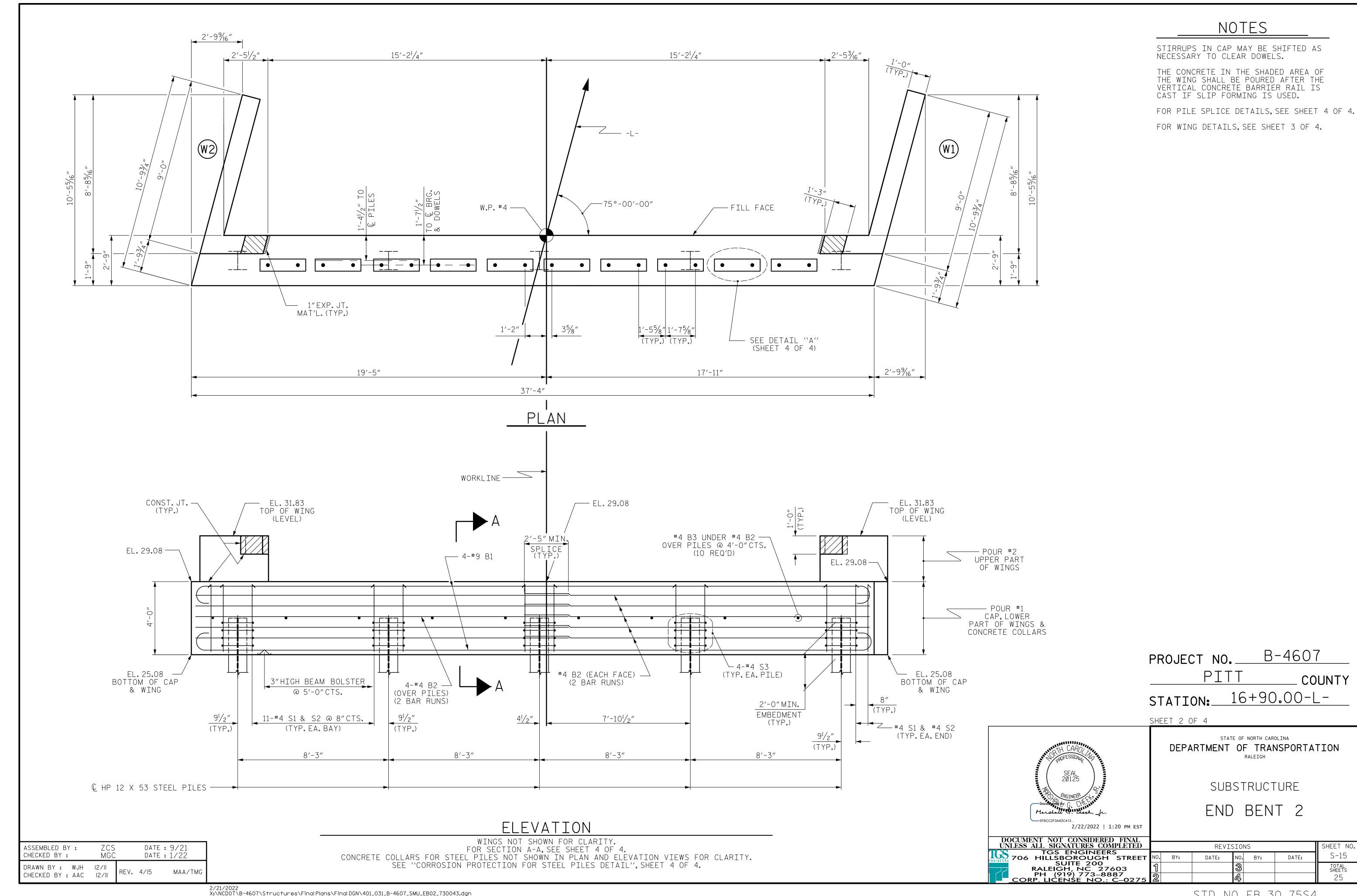
TGS ENGINEERS

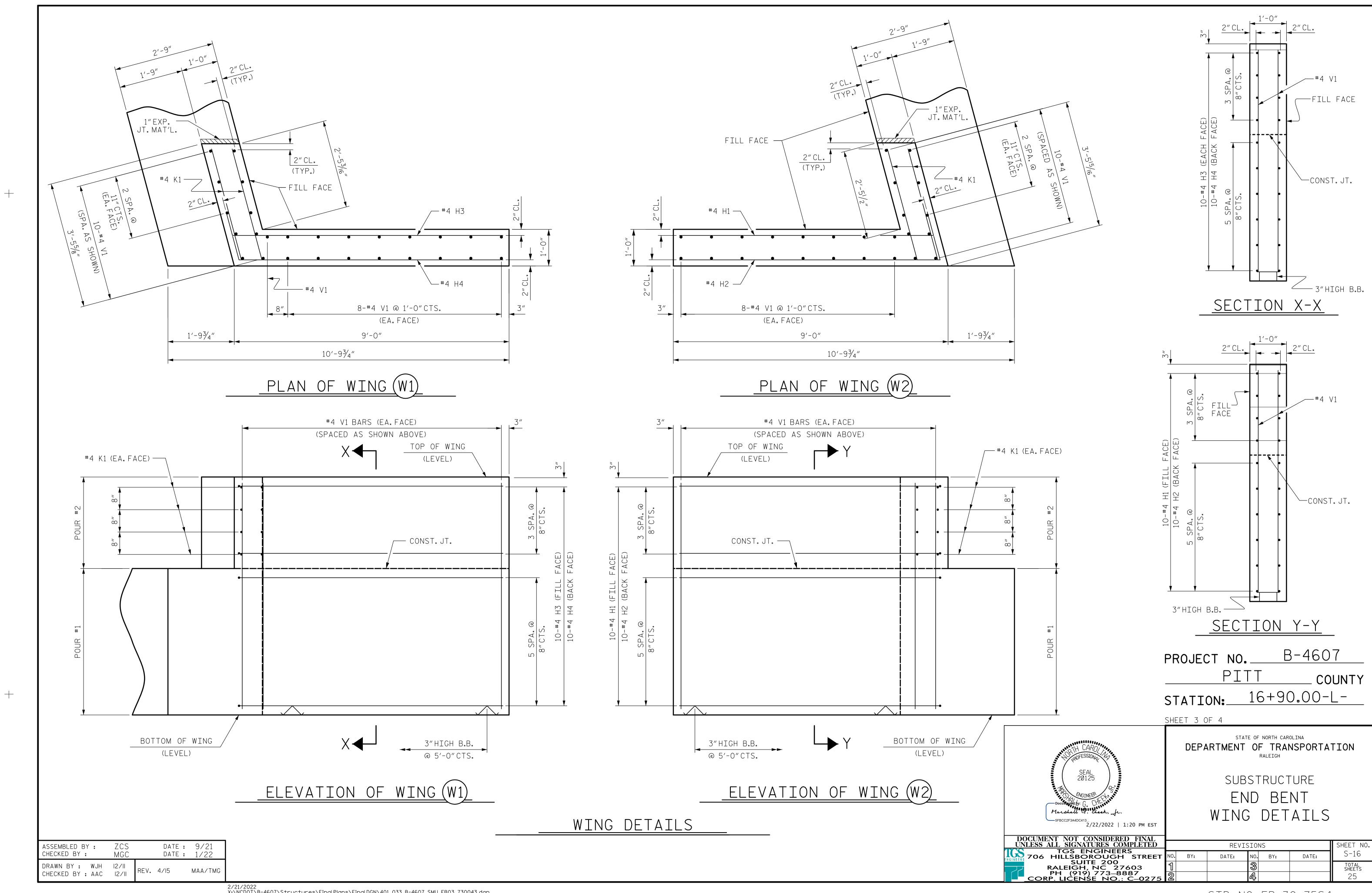
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

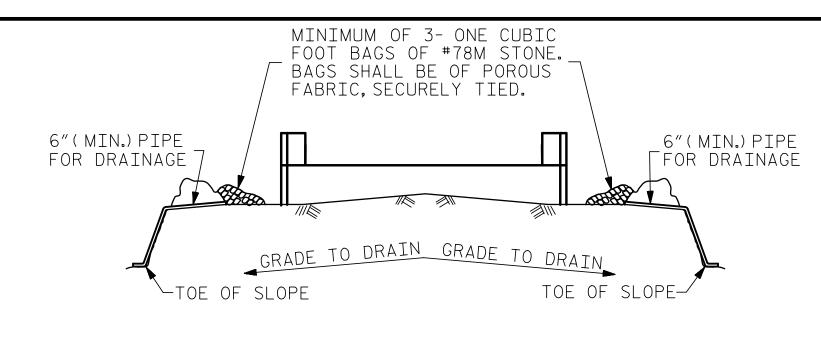
BARRIER RAIL REVISIONS SHEET NO S-13 DATE: BY: DATE: BY: TOTAL SHEETS

DATE: 2/22 DATE: 2/22 ASSEMBLED BY : CHECKED BY : MAA/TMG DRAWN BY: MAA 5/10 MAA/THC CHECKED BY : GM 5/10 MAA/THC







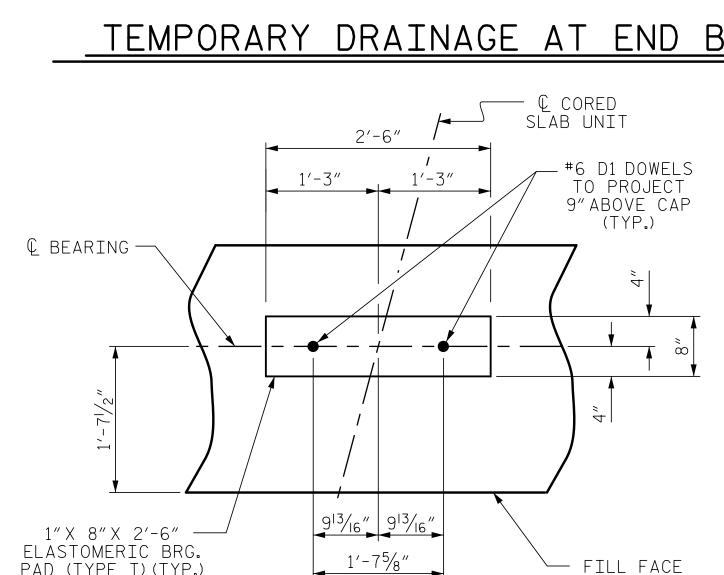


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

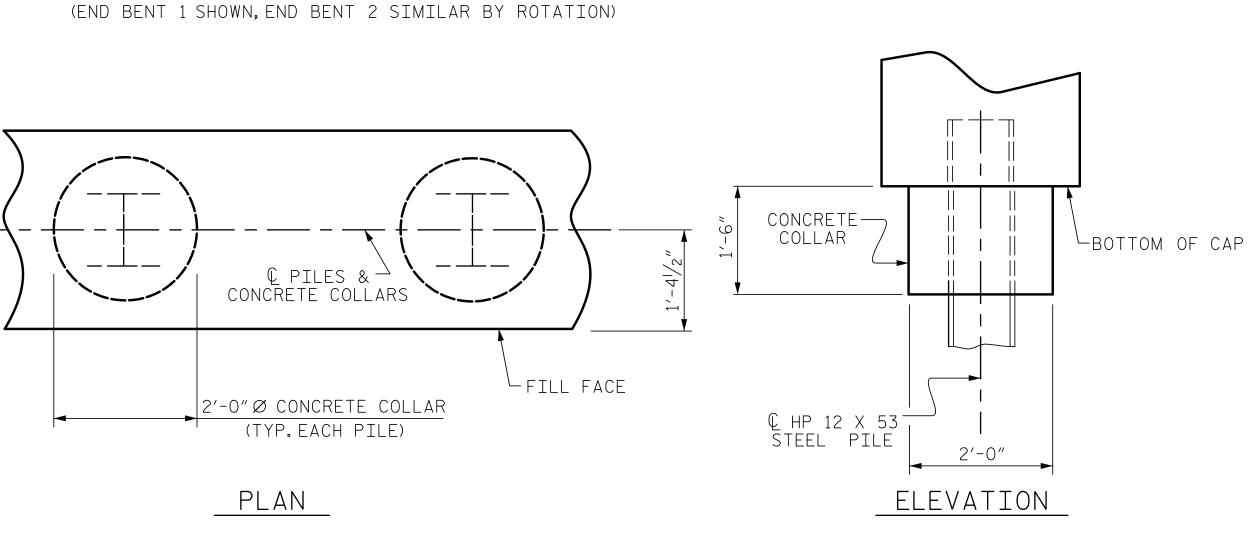
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



DETAIL "A"





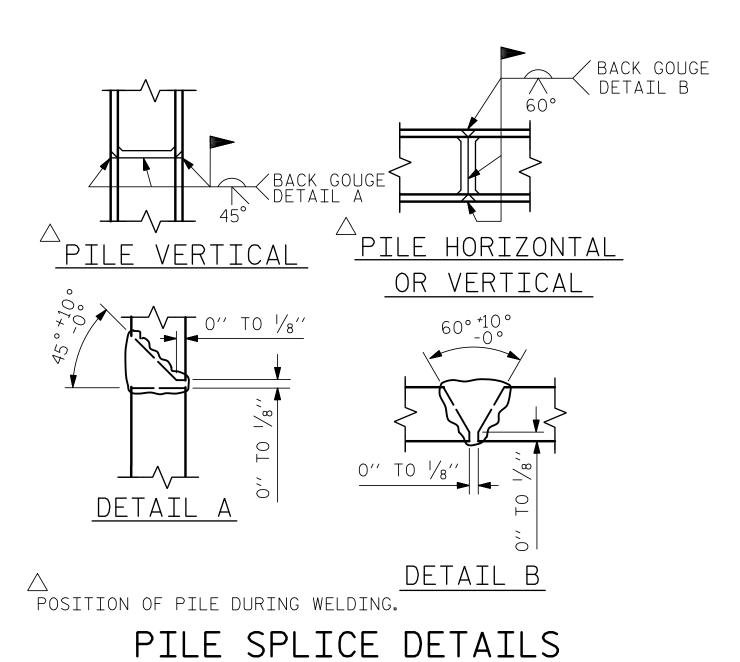
CORROSION PROTECTION FOR STEEL PILES DETAIL

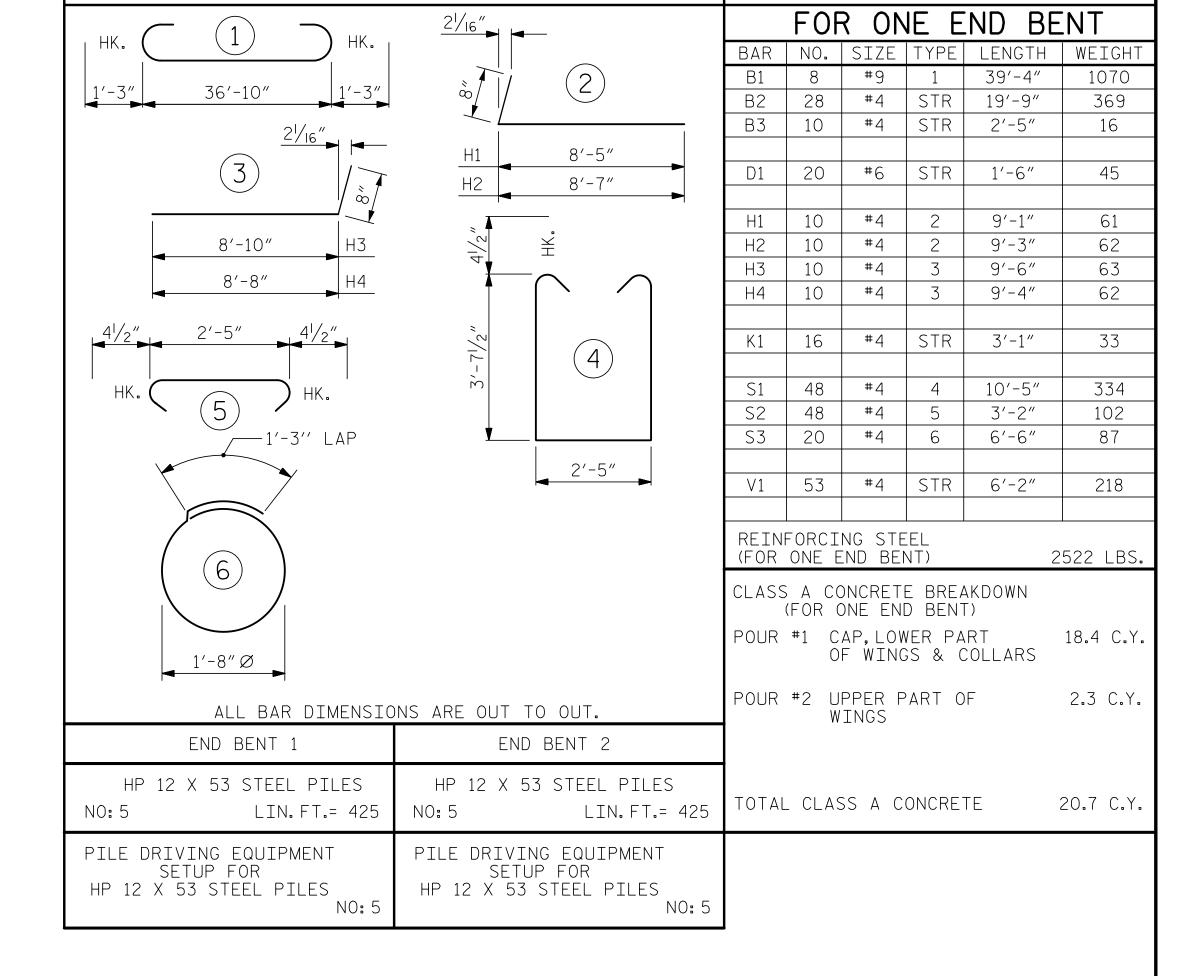
(END BENT 1 SHOWN, END BENT 2 SIMILAR BY ROTATION)

| ASSEMBLED BY : | ZCS | DATE : | 9/21 |
|----------------------------------|-------------|-----------|------|
| CHECKED BY : | MGC | DATE : | 1/22 |
| DRAWN BY: WJH CHECKED BY: AAC | 2/ 2/ | REV. 4/17 | |

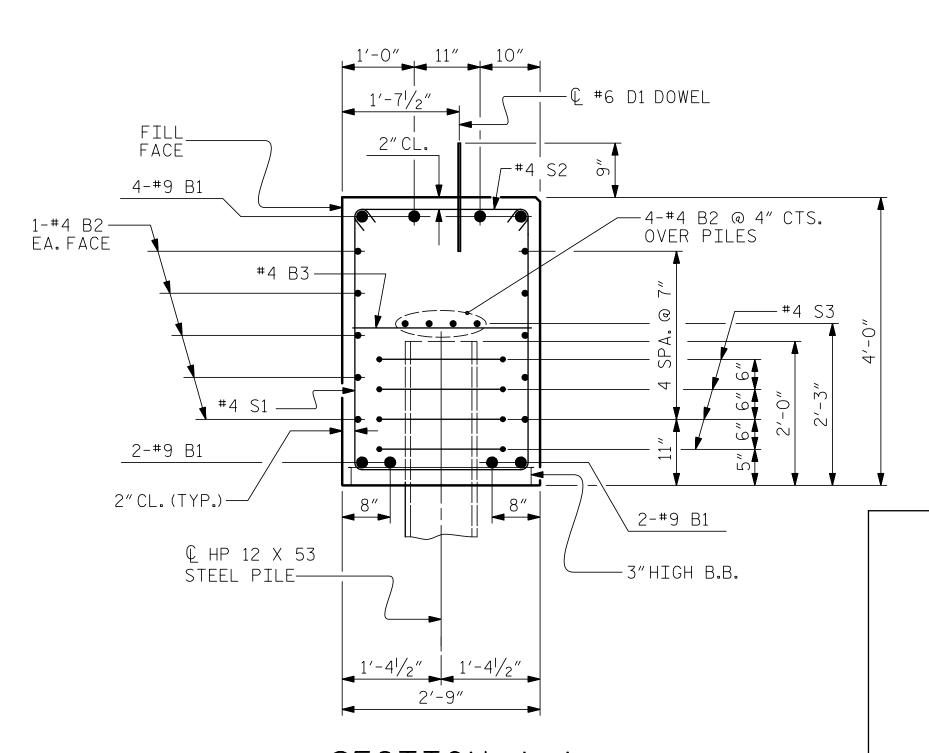
PAD (TYPE I)(TYP.)

+





BAR TYPES



SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL."

B-4607 PROJECT NO. COUNTY 16+90.00-L-STATION:

BILL OF MATERIAL

SHEET 4 OF 4

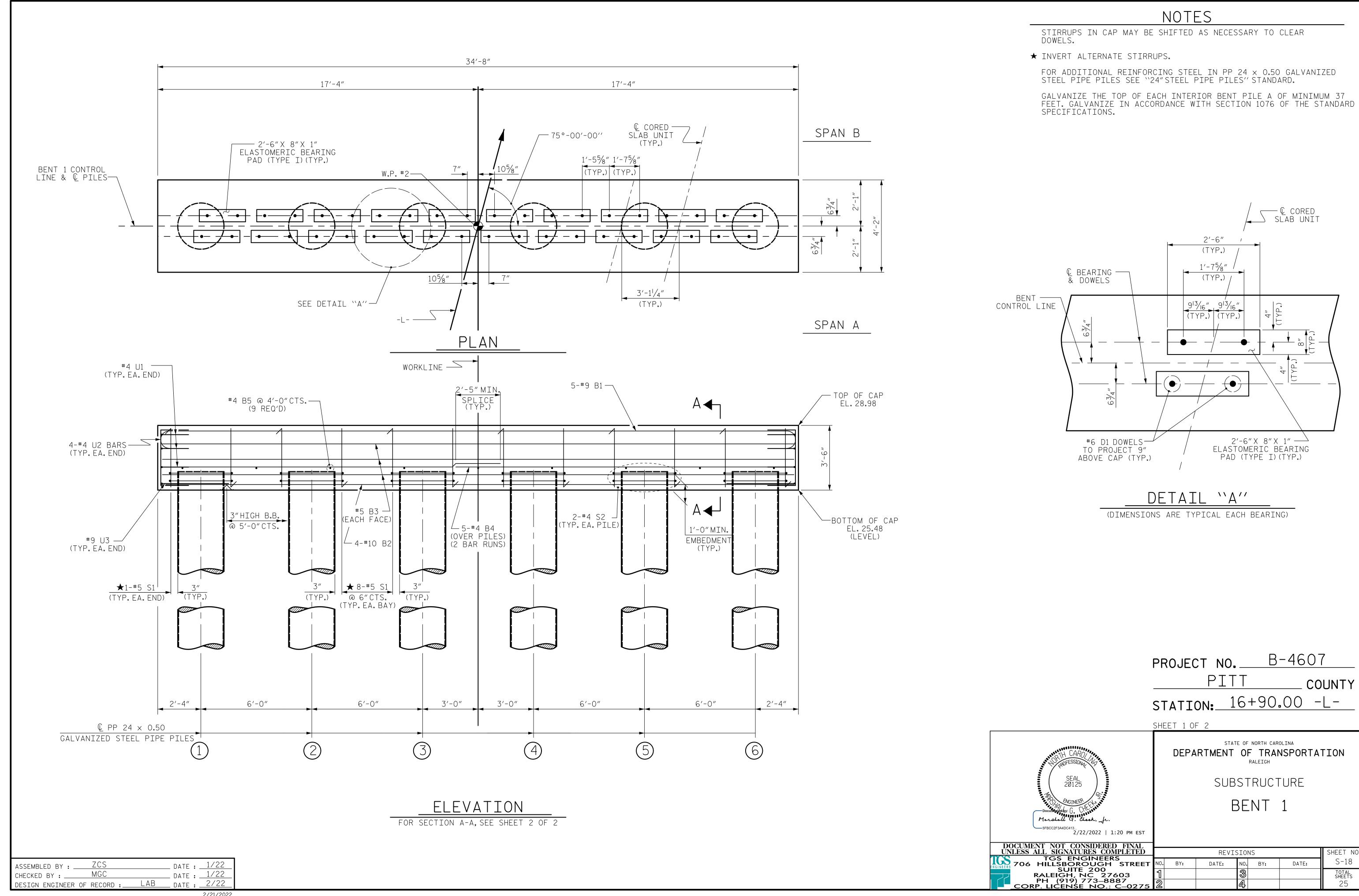
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

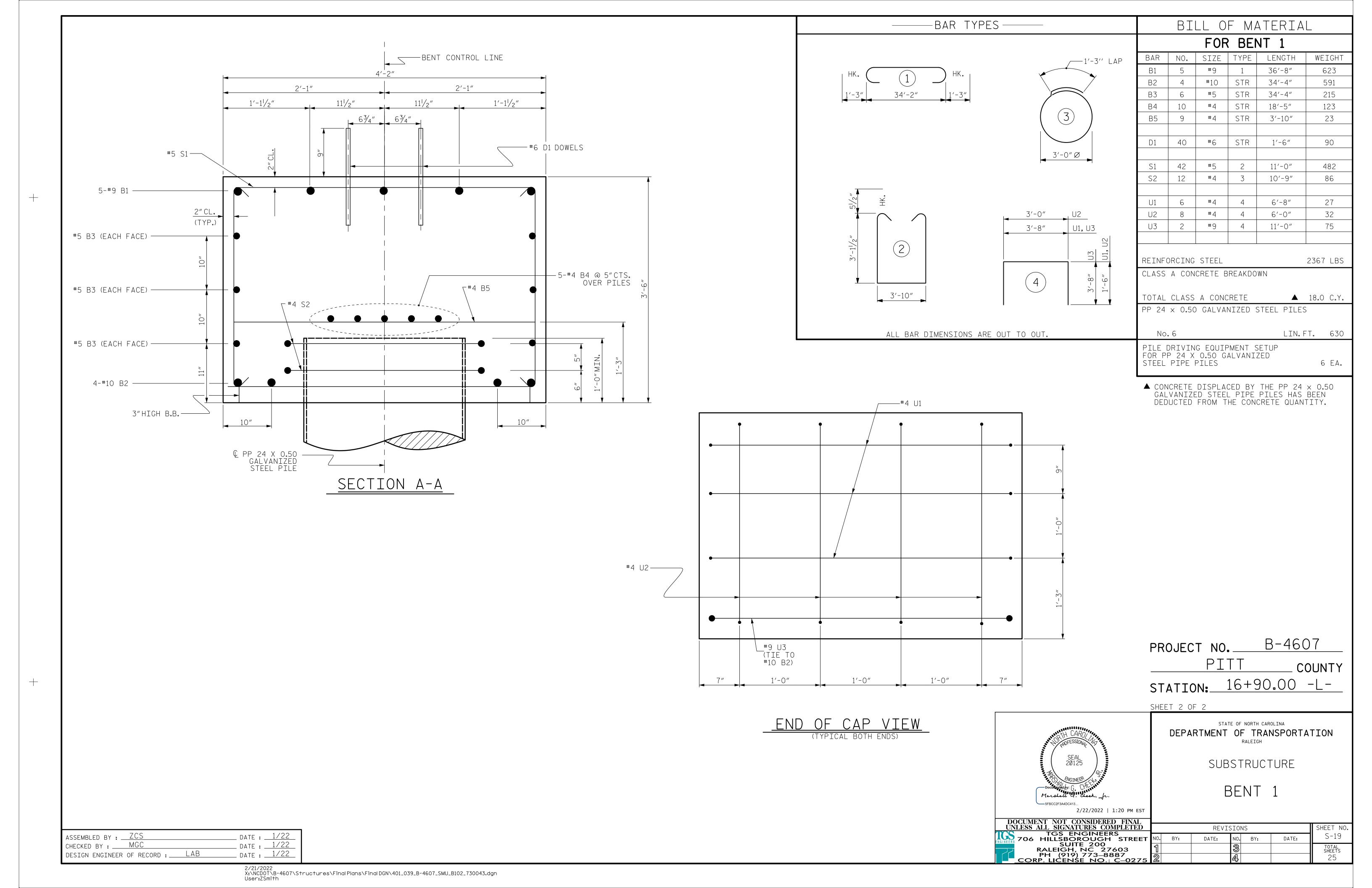
SUBSTRUCTURE

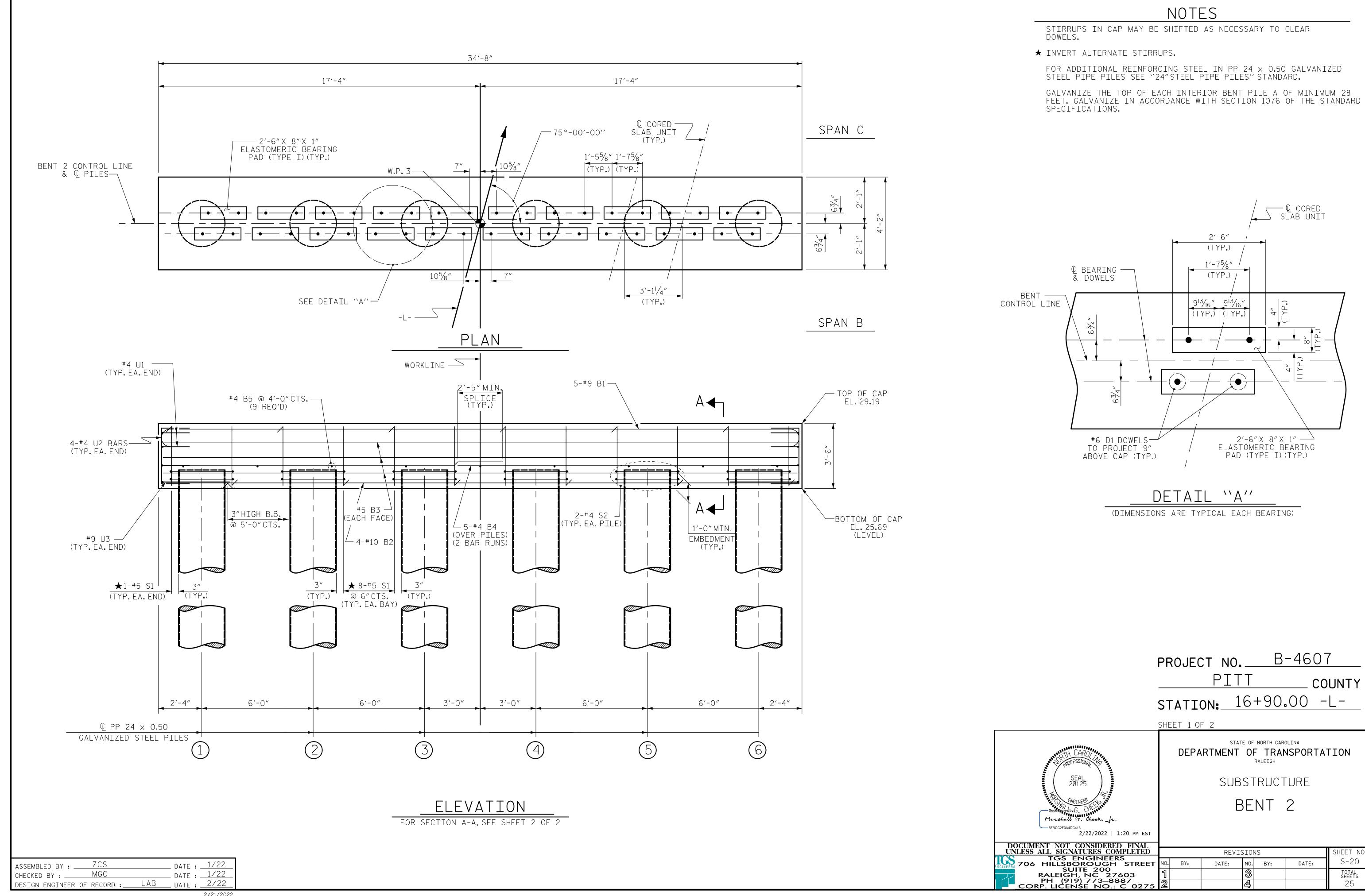
END BENT 1 & 2 DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
706 HILLSBOROUGH STREET SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275 SHEET NO REVISIONS S-17 DATE: BY: DATE: BY: TOTAL SHEETS

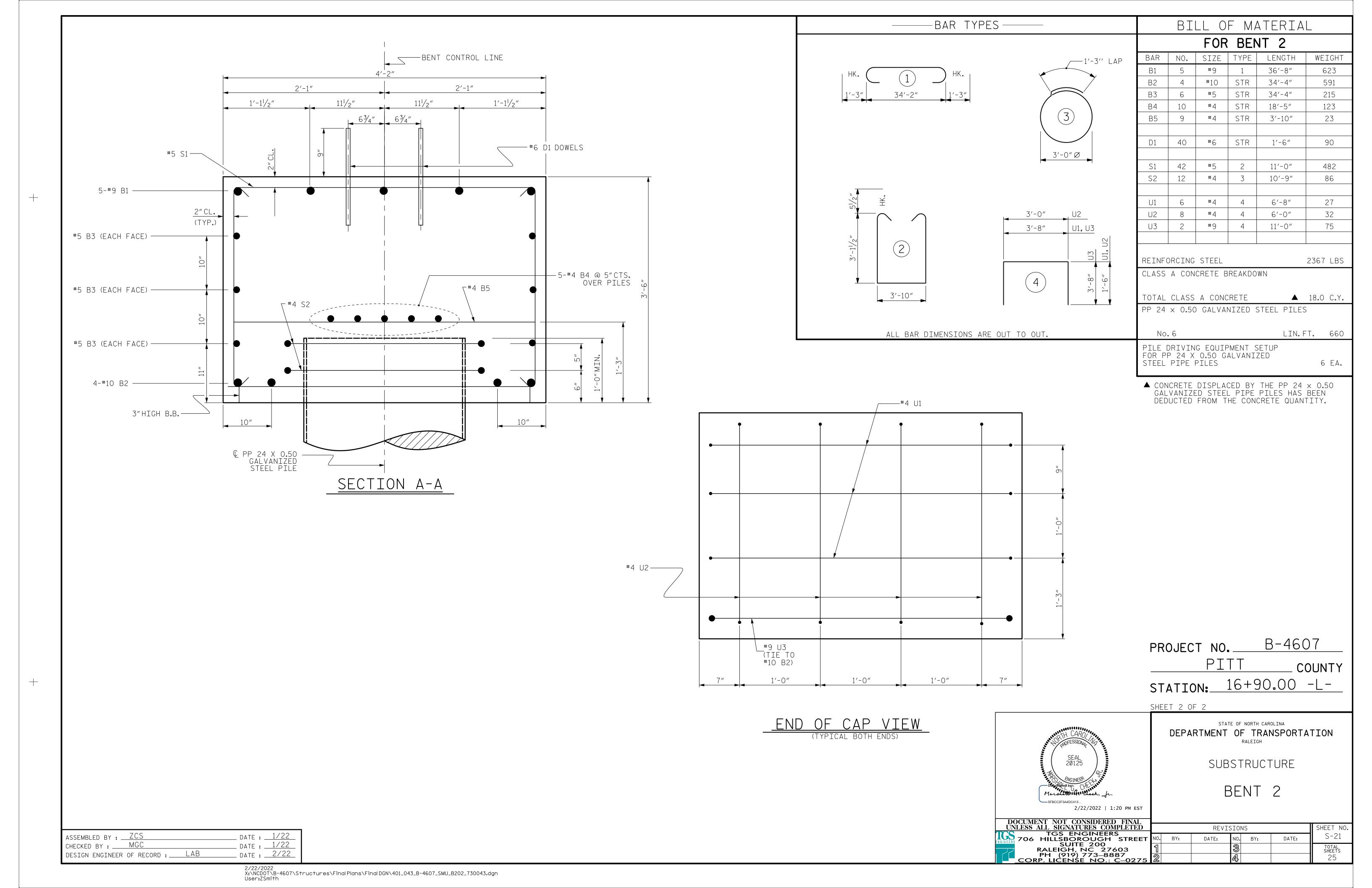


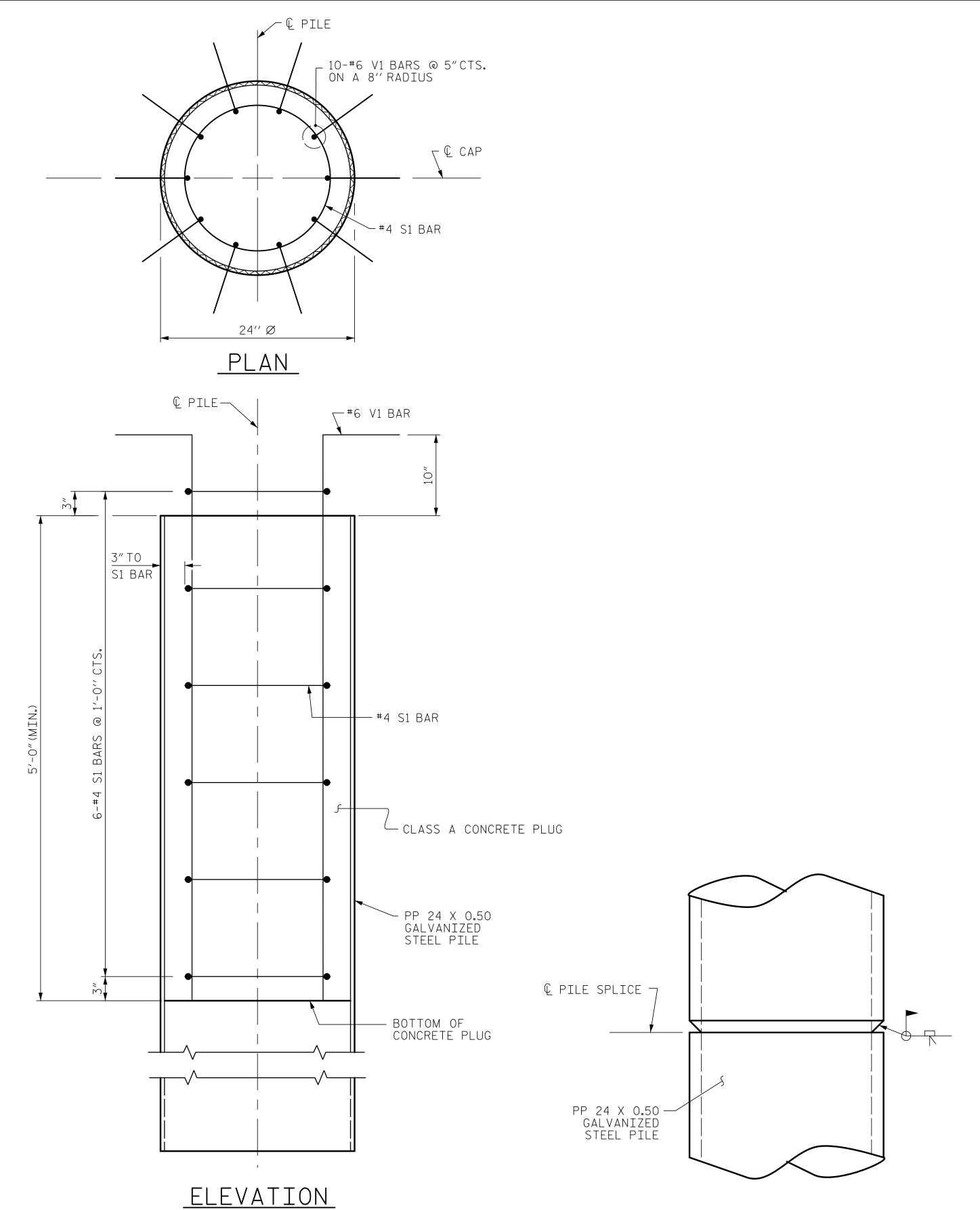




S-20

TOTAL SHEETS





PIPE PILE SPLICE DETAIL

NOTES

PIPE PILES SHALL BE IN ACCORDANCE WITH SECTION 1084 OF THE STANDARD SPECIFICATIONS.

GALVANIZE STEEL PIPE PILES IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS UNLESS METALLIZING IS REQUIRED. GALVANIZING OR METALLIZING PIPE PILE PLATES IS NOT REQUIRED.

PIPE PILE PLATES, IF REQUIRED, SHALL BE IN ACCORDANCE WITH SECTION 450 OF THE STANDARD SPECIFICATIONS.

REMOVE AND REPLACE OR REPAIR TO THE SATISFACTION OF THE ENGINEER PILES THAT ARE DAMAGED, DEFORMED OR COLLAPSED DURING INSTALLATION OR DRIVING.

PILE SPLICES SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND AWS D1.1.

FOR CLOSED END PIPE PILES, REMOVE ALL SOIL AND WATER FROM INSIDE THE PILES JUST PRIOR TO PLACING REINFORCING STEEL AND CONCRETE FOR THE CONCRETE PLUG.

FOR OPEN END PIPE PILES, REMOVE ENOUGH SOIL AND WATER FROM INSIDE THE PILES TO CONSTRUCT THE CONCRETE PLUG WITHOUT FOULING THE CONCRETE.

FORM THE CONCRETE PLUG SUCH THAT THE REINFORCING STEEL OR CONCRETE DOES NOT MOVE AND THE CLEARANCE FROM THE REINFORCING STEEL TO THE INSIDE OF THE PILE IS MAINTAINED AFTER CONCRETE PLACEMENT. DO NOT PLACE CONCRETE IN THE BENT CAP UNTIL THE CONCRETE PLUG HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 1500 PSI.

THE REINFORCING STEEL, CLASS A CONCRETE, AND GALVANIZING ARE CONSIDERED INCIDENTAL TO THE CONTRACT UNIT PRICE BID PER LINEAR FOOT FOR PP 24 X 0.50 GALVANIZED STEEL PILES.

BILL OF MATERIAL FOR ONE PP 24 X 0.50 GALVANIZED STEEL PILE

NO. | SIZE | TYPE LENGTH 6 #4 6'-0'' S1 24 100 10 #6 6'-8'' V1 REINFORCING STEEL = 124 lbs

CLASS A CONCRETE

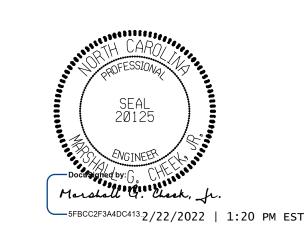
0.5 CY 5'-0'' MINIMUM PLUG

BAR TYPES

---1'-3'' LAP 5'-10'' 1'-6'' ALL BAR DIMENSIONS ARE OUT TO OUT.

B-4607 PROJECT NO._ COUNTY

STATION: 16+90.00 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

24" STEEL PIPE PILE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS 804–C N. LAFAYETTE ST SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275

REVISIONS DATE: DATE: BY:

SHEET NO

S-22

TOTAL SHEETS

ZCS MGC

ASSEMBLED BY :

DRAWN BY: TLA 8/05

CHECKED BY: GM 9/05

CHECKED BY :

DATE: 1/22 DATE: 1/22

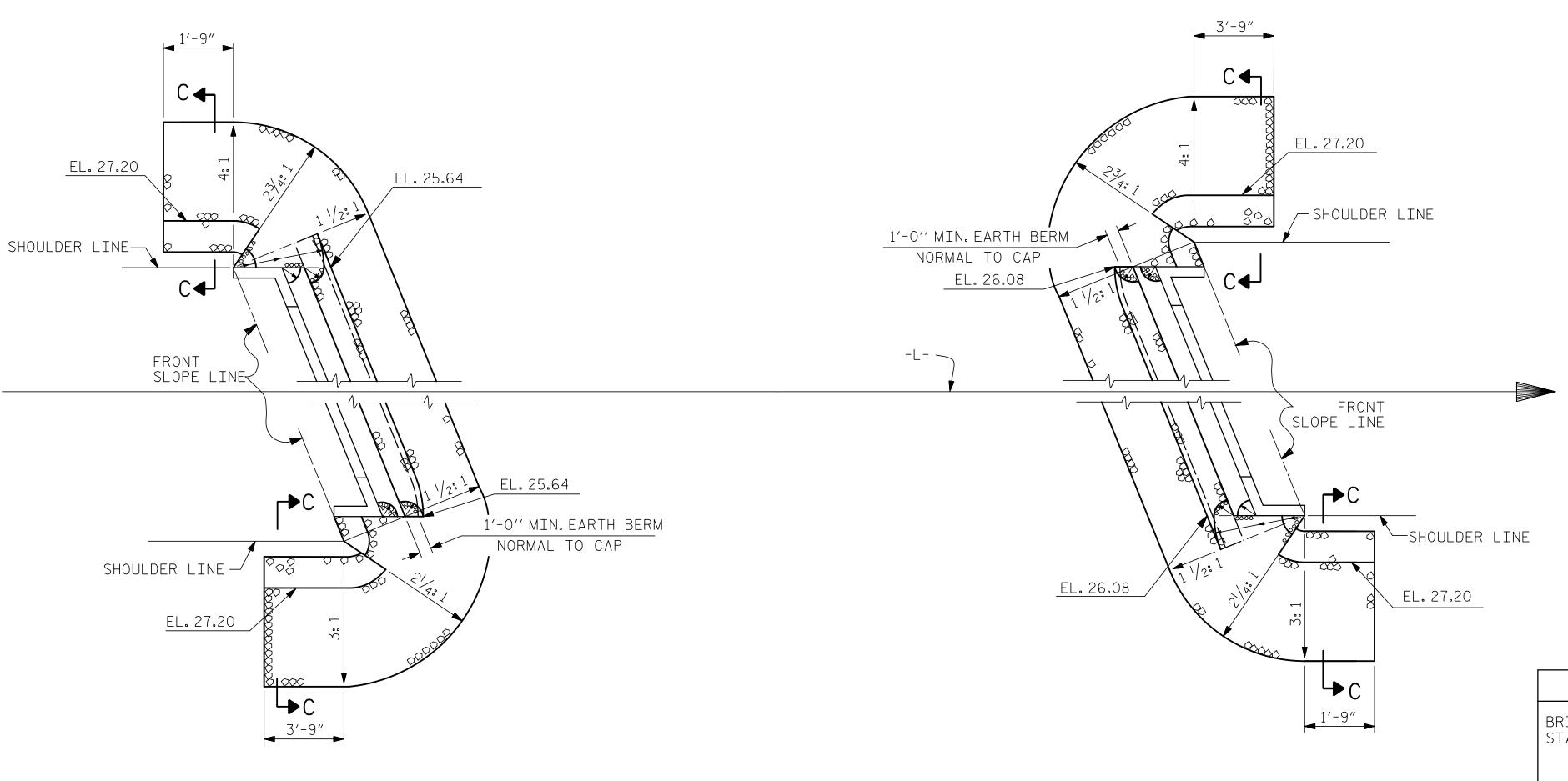
MAA/GM

MAA/THC

PP 24 X 0.50 GALVANIZED STEEL PILE

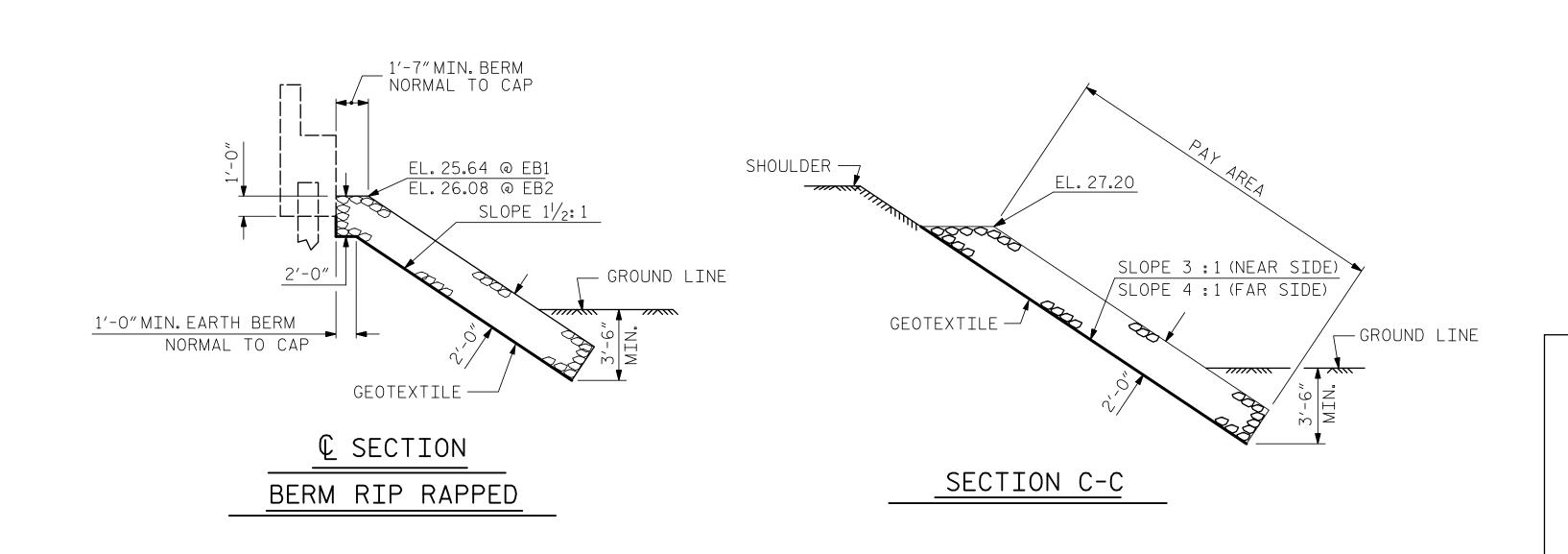


FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.



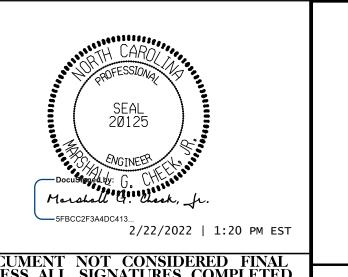
PLAN

| ESTIMATED QUANTITIES | | | |
|-----------------------------|-------------------------------------|----------------------------|--|
| BRIDGE @ STA.16+90.00-L- | RIP RAP CLASS II (2'-0"THICK) | GEOTEXTILE FOR DRAINAGE | |
| | TONS | SQUARE YARDS | |
| END BENT 1 | 160 | 180 | |
| END BENT 2 | 190 | 210 | |



END BENT 2

PROJECT NO. B-4607
PITT COUNTY
STATION: 16+90.00-L-



DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

STANDARD

RIP RAP DETAILS

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TGS ENGINEERS
706 HILLSBOROUGH STREET
SUITE 200
RALEIGH, NC 27603
PH (919) 773–8887
CORP. LICENSE NO.: C-0275

REVISIONS

SHEET NO.
BY: DATE: NO. BY: DATE: NO. BY: DATE: S-23

CORP. LICENSE NO.: C-0275

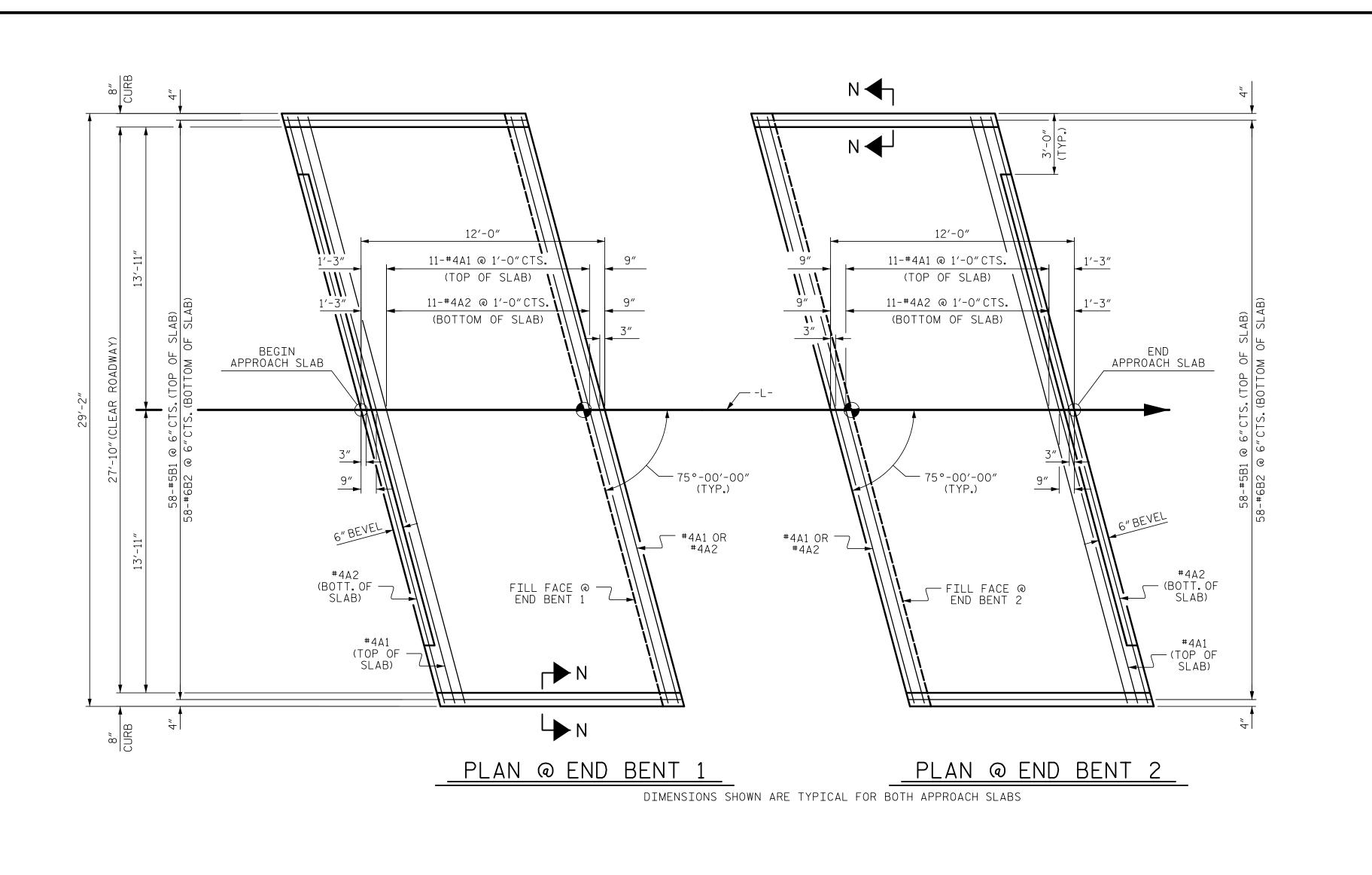
ASSEMBLED BY: ZCS DATE: 9/21 CHECKED BY: MGC DATE: 1/22

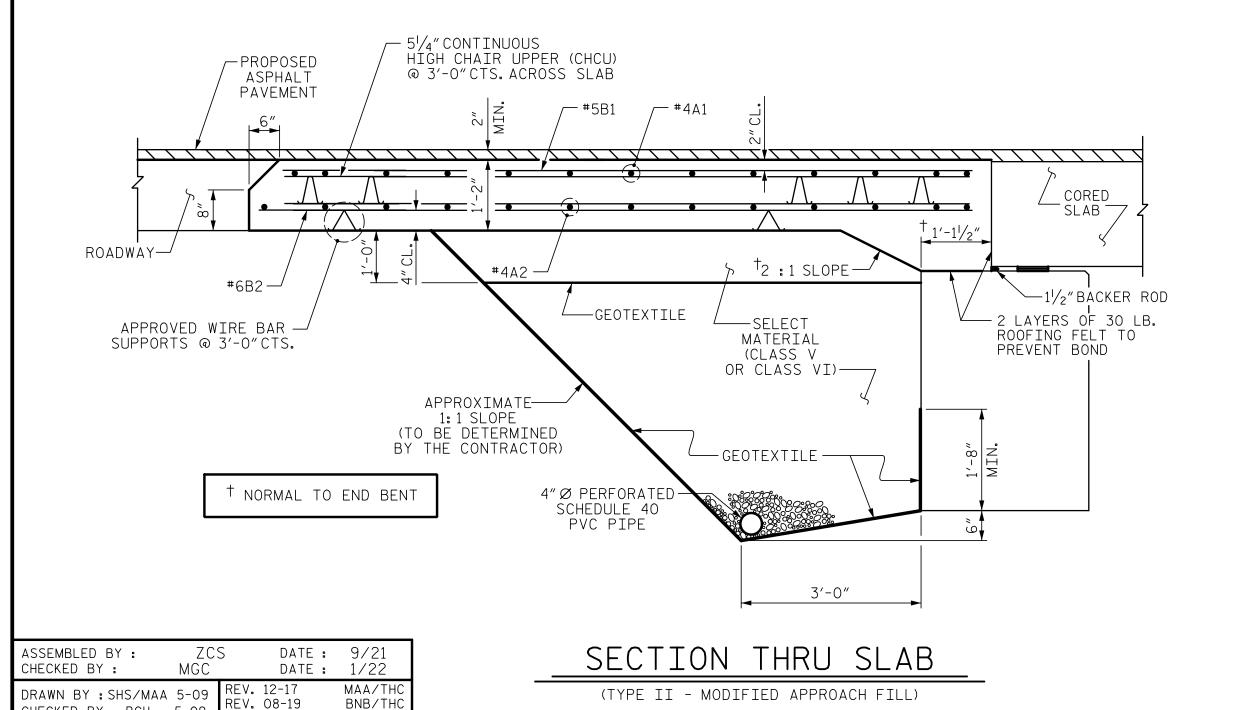
DRAWN BY: REK 1/84 REV.10/1/II MAA/GM REV.12/21/II MAA/GM REV.12/17 MAA/THC

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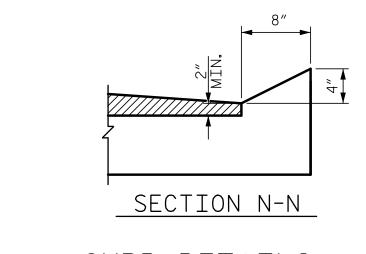
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END BENT 1





CHECKED BY : BCH 5-09



CURB DETAILS

| SPLICE LENGTHS | | |
|----------------|-----------------|----------|
| BAR SIZE | EPOXY COATED | UNCOATED |
| #4 | 1'-11" | 1'-7" |
| #5 | 2′-5″ | 2'-0" |
| #6 | 3′-7″ | 2'-5" |

NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

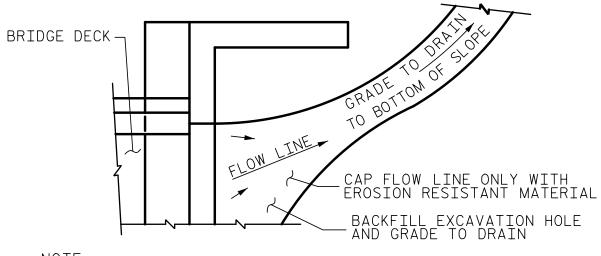
SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL

APPROACH SLAB GROOVING IS NOT REQUIRED.

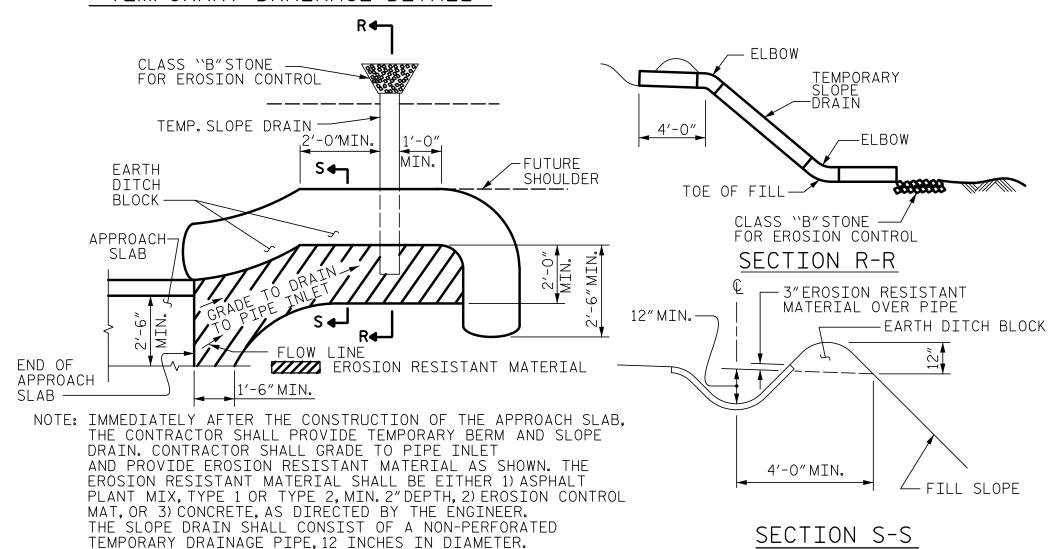
BE PAVED. SEE ROADWAY PLANS.



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PLAN VIEW



TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

PROJECT NO. B-4607

PITT COUNTY

STATION: 16+90.00-L-

BILL OF MATERIAL

APPROACH SLAB AT EB 1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

259

1009

1268

259

1009

1268

LBS.

LBS.

C.Y.

LBS.

LBS.

C.Y.

* A1 | 13 | #4 | STR | 29'-10"

*B1 | 58 | #5 | STR | 11'-1"

REINFORCING STEEL

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

* EPOXY COATED

* EPOXY COATED

A2 | 13 | #4 | STR | 29'-10"

58 | #6 | STR | 11'-7"

APPROACH SLAB AT EB 2

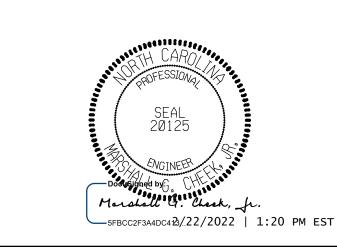
BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

* A1 | 13 | #4 | STR | 29'-10"

*B1 | 58 | #5 | STR | 11'-1"

B2 | 58 | #6 | STR | 11'-7"

A2 | 13 | #4 | STR | 29'-10"



DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

BRIDGE APPROACH SLAB
FOR PRESTRESSED CONCRETE
CORED SLAB UNIT
(SUB-REGIONAL TIER)

SHEET NO

S-24

TOTAL SHEETS

CONSIDERED FINAL
ATURES COMPLETED

REVISIONS

ENGINEERS
LAFAYETTE ST

REVISIONS

DATE: NO. BY: DATE:

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
804-C N. LAFAYETTE ST
SHELBY, NC 28150
PH (704) 476-0003
CORP. LICENSE NO.: C-0275

-C N. LAFAYETTE ST HELBY, NC 28150 H (704) 476–0003 LICENSE NO.: C-0275

STANDARD NOTES

DESIGN DATA:

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED \(\frac{3}{4}\) WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1\(\frac{1}{2}\) RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A \(\frac{1}{4}\) FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A \(\frac{1}{4}\) RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

<u>Allowance for dead load deflection, settlement,</u>

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/6" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY /16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

PROJECT NO. B-4607
PITT COUNTY
STATION: 16+90.00-L-

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD NOTES

TGS ENGINEERS
804-C N. LAFAYETTE ST
SHELBY, NC 28150
PH (704) 476-0003
CORP. LICENSE NO.: C-0275

REVISIONS SHEET NO.

BY: DATE: NO. BY: DATE: S-25

3 TOTAL SHEETS
25